G. Recreation

The Loop, with both soft- and hard-surface trails, is located within the Rillito River Park adjacent to the Property along the south. There are multiple trailheads located along the Rillito River Park, including Craycroft Trailhead, which is adjacent to the Property on the east. (See *Exhibit IV.G: Recreation Facilities.*) Other trailheads within one mile of the Property are indicated in Table IV.G, below.

Table IV.G: Trailheads & Trail Access Points

	Approximate Distance
Trailheads & Trail Access Points	from Property (feet)
Craycroft Trailhead	<100
Craycroft SW Access Point	890
Craycroft SE Access Point	940
Hill Farm & Hill Ranch Access Points	1100
Swan South Trailhead	3100
Swan SW Access Point	3300
Swan North Access Point	3800
Greenlee Road Access Point	4600

Fort Lowell Park is located approximately 2700 feet southeast of the Property along Craycroft Road.

H. Schools

The Project is located in Tucson Unified School District ("TUSD"). As indicated in correspondence from TUSD, the school district has capacity to accommodate student from the projected new homes in Parcel B. (See *Exhibit IV.H: TUSD Capacity Response.*)

There are no schools abutting the Property. Gregory School and Castlehill Country Day School, identified on *Exhibit IV.G*, are located south of the Property, south of the Rillito Creek. BASIS School is located northeast of the Property within one mile.

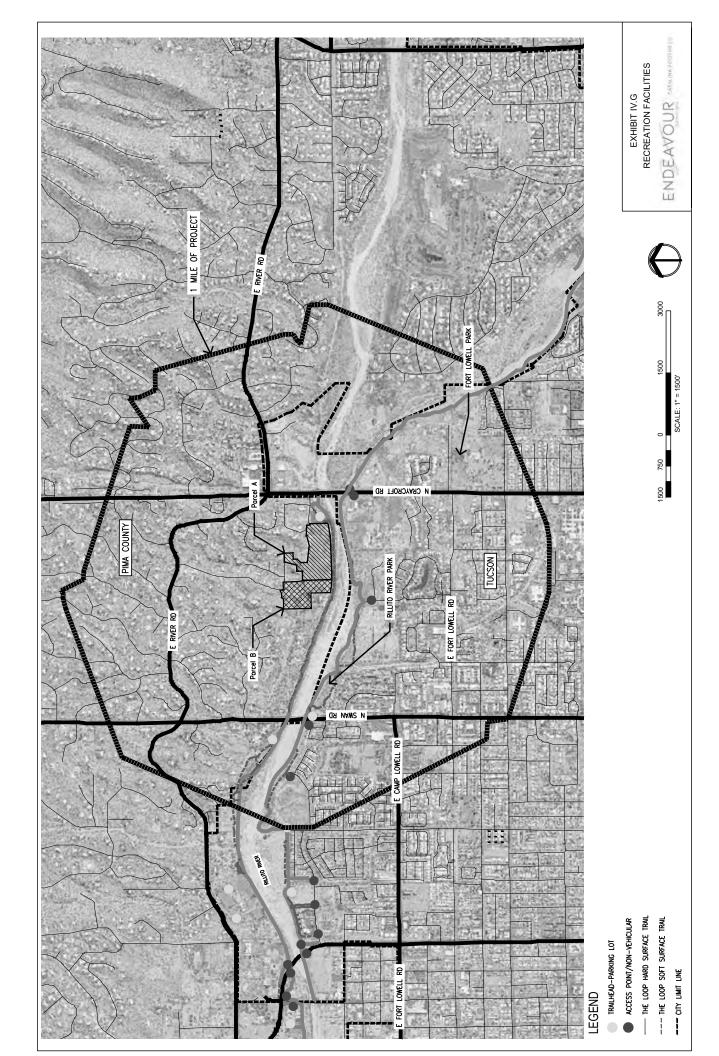


Exhibit IV.H: TUSD Capacity Response

TUCSON UNIFIED

Department of Engineering, Facilities and Planning
TUSD Planning Services – 2025 E Winsett Street Tucson, Arizona 85719
(520) 225-4949
(520) 225-4939 (fax)

To: Robin Large

Senior Land Planner

From: Shaun Brown

District Planner

Date: December 20, 2021

Re: Case/Project #:

Project Name: Endeavour Spirited Living Specific Plan

New Units: 2

Impacted Schools	Capcity	Projected Enrollment 2020	Additional Students from Project	Projected Enroll w/Project	Students Exceeding Capacity	Students Exceeding Capacity %
Whitmore Elementary	490	318	6	324	-166	-34%
Doolen Middle	1140	586	3	589	-551	-48%
Catalina High	1500	618	4	622	-878	-59%

Response:

Based on the projected enrollment at TUSD, there is adequate capacity to absorb the impact of the proposed 22 single-family residence at Whitmore Elementary, Doolen Middle and Catalina High School.

I. Cultural Resources

Per consultation with the Arizona State Museum ("ASM"), multiple cultural resource surveys have been conducted within one mile of the Property and two within portions of the Parcel A. One archaeological site was identified on the Property. ASM recommends the Property be resurveyed prior to ground-disturbing activities since the last survey was conducted more than 10 years ago.

Correspondence from the Arizona State Museum is included as Exhibits IV.I.1 & 2.

J. Composite

Composite Maps are provided for both Parcel A and Parcel B. (See *Exhibits IV.J.1 & 2*.) These maps indicate the Property's existing physical constraints, including structures, topography, riparian habitat, utilities and easements.

Exhibit IV.I.1: Parcel A Arizona State Museum Records Check



Arizona State Museum PO Box 210026 Tucson AZ 85721-0026 (520) 621-6281 www.statemuseum.arizona.edu

October 20, 2020

Robin Large Lazarus & Silvyn, P.C. 5983 E. Grant Rd., Stc. 290 Tueson, AZ 85712

RE: Endeavor Specific Plan Parcels 109-26-003H, 109-26-003D, 109-26-004C, 109-26-004D

Dear Robin,

Arizona State Museum (ASM) has reviewed archaeological project and site records in support of project: Endeavor Specific Plan. Correspondence indicates this project will involve the rezoning of private-owned land with the intent of developing an active adult community. The project area is located at 5240 E. River House Rd., 3475 N. Craycroft Rd., and 3505 N. Craycroft Rd. in the city of Tucson, Pima County, and encompasses parcels 109-26-003H, 109-26-003D, 109-26-004C, 109-26-004D within Township 13 South, Range 14 East, Section 26. Below are the results of ASM's research.

Search Results:

According to a search of the archaeological site files and records retained at ASM, 51 archaeological survey projects were conducted within a one-mile radius of the project area between 1979 and 2015. Previous survey work was conducted in support of:

- residential and commercial development;
- installation of gas, water, sewage, cable, and fiber optic lines;
- water recharge development;
- road widening, grading, clearance, and improvements;
- riverbank protection/clearance and wash/river channelizing;
- soil testing;
- water hydrant clearance;
- installation of street lights:
- development of a communications tower and facility;
- installation of cellular telephone booster nodes;
- · fence installation on the perimeter of a ballpark and park improvements;
- · hospital expansion; and
- sidewalk installation and improvement.

Two archaeological survey projects have been conducted within a portion of the project area. These previous surveys were conducted in support of water recharge development (ASM Accession #1989-121; Hohmann 1989) and riverbank clearance (ASM Accession #1994-087; Czaplicki 1994). These surveys did not identify any archaeological sites within the project area.

Twenty-six archaeological sites have been identified within a one-mile radius of the project area, one of which is within the current project area (AZ BB:9:302 [ASM]).

Recommendations and Responsibilities:

- 1. Although portions of the project area have been previously surveyed, the work was conducted 21 and 26 years ago, respectively, it is standard archaeological practice for a property to be re-surveyed if the previous survey was conducted 10 or more years ago, as there is a possibility for unidentified archaeological properties to have since been exposed. Therefore, ASM recommends, but it is not required by ASM, that a qualified archaeological contractor be consulted before any ground-disturbance begins. A list of archaeological contractors is available on the ASM website at: https://statemuseum.arizona.edu/crm
- Pursuant to Arizona Revised Statute §41-865, if any human remains or funerary objects are discovered during project work, all work will stop within the area of the remains and ASM's Repatriation Office will be contacted at 520-626-2950.
- City, county, or municipal governments may have requirements, therefore, ASM recommends that the relevant jurisdiction(s) be consulted.

If you have any questions about the results of this records search, please feel free to contact me twilling@email.arizona.edu or 520-621-4795.

Sincerely.

Shannon Twilling, M.A.

Arizona Antiquities Act Administrator

Arizona State Museum

References:

Czaplicki, Jon S.

1994 Archaeological Survey of a Portion of the Rillito River Bank Between Swan and Craycroft Roads, Pima County, Arizona. Bureau of Reclamation, Arizona Projects Office, Phoenix.

Hohmann, John W.

1989 A Phase I Archaeological Reconnaissance of the Proposed Rillito Creek Recharge Site for Camp Dresser & McKee, Inc., Tucson, Arizona. The Cultural Resource Group, Louis Berger and Associates, Inc., Phoenix.

Exhibit IV.I.2: Parcel B Arizona State Museum Records Check



Arizona State Museum PO Box 210026 Tucson AZ 85721-0026 (520) 621-6281 www.statemuseum.arizona.edu

August 9, 2021

Robin Large Lazarus & Silvyn, P.C. 5983 E. Grant Rd., Ste. 290 Tucson, AZ 85712

RE: Endeavour Spirited Living Specific Plan

Parcels 109-26-005H, 109-26-005L, and 109-26-005M

Dear Robin,

The Arizona State Museum (ASM) has reviewed archaeological project and site records in support of project: Endeavour Spirited Living Specific Plan. Correspondence indicates this project will involve the rezoning and development of privately-owned land with the intent of constructing residential housing. The project area is located at 5150 E. River House Road in the city of Tucson, Pima County, and encompasses parcels 109-26-005H, 109-26-005L, and 109-26-005M within Township 13 South, Range 14 East, Section 26. I invite you to review the results of ASM's research, which are summarized below.

Search Results:

According to a search of the archaeological site records and reports held in ASM collections, 80 archaeological investigations were conducted within a one-mile radius of the project area between 1979 and 2019. Five (5) of these archaeological investigations intersect the current project area.

For the five (5) archaeological investigations that intersect the project area, Table 1 summarizes their basic information and scope:

Table 1. ASM archaeological investigations that intersect the project area

ASM Reference Number (AZProj/Accession)	Report Author	Year Conducted	Scope of Project		
AZPROJ 1979-0031/ AP-1979-0203	J. Schaefer and S. A. Brew; S. A. Brew	1979	Survey for archaeological clearance		
AZPROJ 1989-0002	Torrello and J. A. 1990		Survey for proposed river channelization clearance		
AZPROJ 1989-0121	J. W. Hohmann	1989	Survey for water recharge complex		
AZPROJ 1994-0087	J. S. Czaplicki	1994	Survey for riverbank clearance		
AP-2013-0522 J. Howell and C. Coppersione		2013	Survey for sewer line modification/enhancement		

Additionally, 28 archaeological sites have been identified within a one-mile radius of the project area, one (1) of which intersects the current project area. The following site number intersects the project area: AZ. BB:9:19(ASM).

Recommendations and Responsibilities:

- Since a portion of the project area has not been subject to prior archaeological survey, ASM recommends—but does not require—that a qualified archaeological contractor be consulted before any ground-disturbing activity begins. A list of archaeological contractors is available on the ASM website at: https://statemuscum.arizona.edu/crm/document/aaa-qualified-consultants
- Pursuant to Arizona Revised Statute §41-865, if any human remains or funerary objects are discovered during project work, all work must stop within the area of the remains and the ASM Repatriation Office must be contacted at 520-626-0320.
- 3. City, county, or municipal governments may have their own requirements; therefore, ASM recommends that the relevant jurisdiction(s) be consulted.

If you have any questions about the results of this records search, please feel free to contact me at eebornemann@arizona.edu or 520-621-4011.

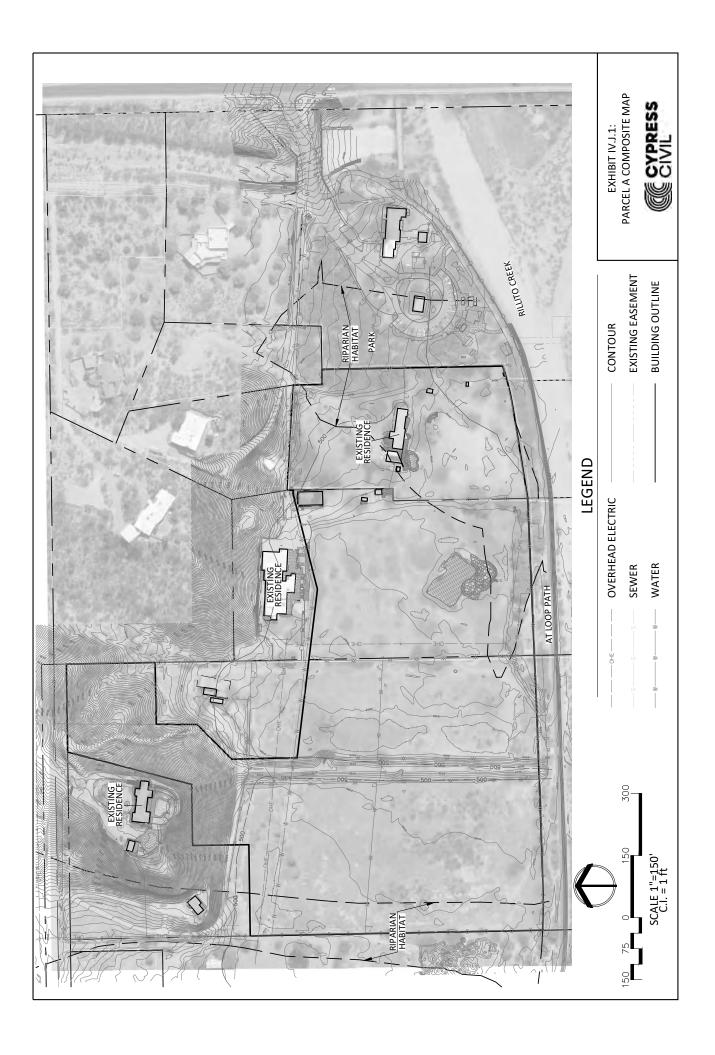
Best regards,

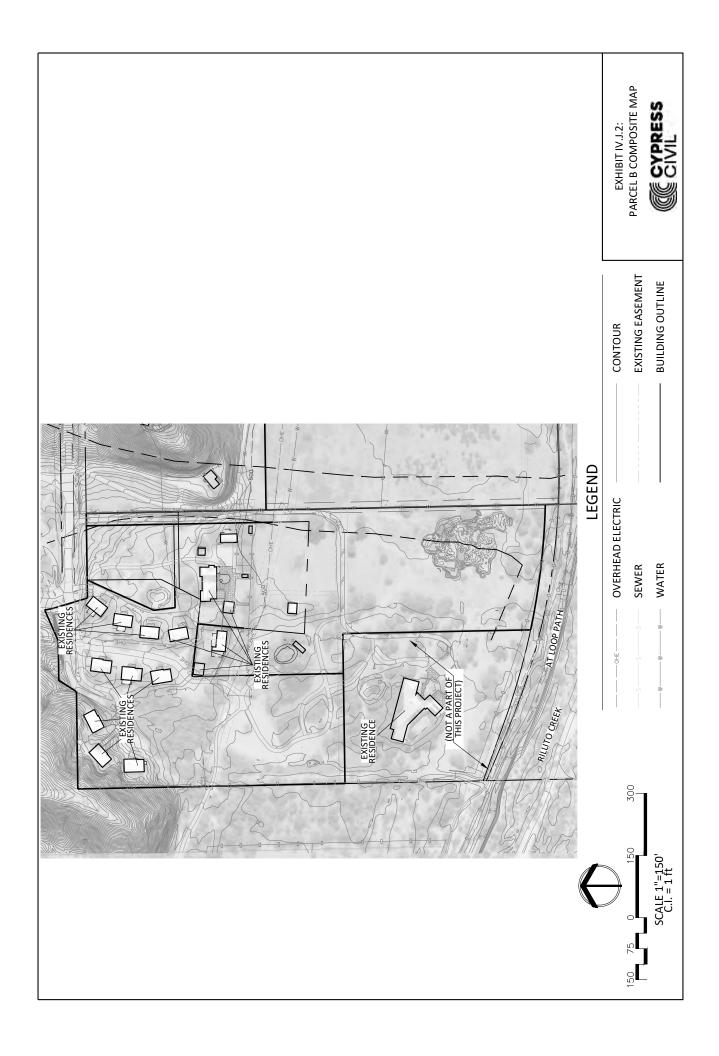
Erin Bornemann, Ph.D., RPA

Research Specialist

Archaeological Records Office

Arizona State Museum





- V. CONDITIONS OF APPROVAL
 - A. Purpose
 - B. Board of Supervisors Conditions of Approval

VI. REFERENCES

Pima County. "PimaMaps." Pima County, Arizona, http://gis.pima.gov/pimamaps/.

Sterner, Matthew A. *"Schroeder's Well and the Davidson Flume: A Glimpse into Tucson's Mormon Culture."* Technical Report No. 96-5. Statistical Research, Inc. 1995. Tucson, Arizona.

Appendix A

Endeavour Spirited Living Conservation Land System Important Riparian Area Analysis



Specific Plan Parcels A and B

Conservation Land System Analysis
For Important Riparian Area (IRA)
Prepared by
Novak Environmental, Inc.
Karen Cesare, RLA, ASLA

Original: August 28, 2020
Updated to include Parcel B: June 29, 2021
Revised: May 25, 2022

I. Introduction

Envisage Living Communities ("Envisage") is proposing to develop Endeavour Spirited Living, which is an active adult independent living project (The Endeavour Project) west of Craycroft Road and south of River Road, referred to hereafter as "Parcel A". Parcel A includes four parcels (Pima County Assessor Parcel Numbers 109-26-003D, -004C, -004D and -003H) and encompasses approximately 28 acres, is currently within unincorporated Pima County ("County") and is zoned County Suburban Ranch ("SR"). Approximately 20 acres of Parcel A will be rezoned to Specific Plan ("SP") to support development of this unique Project. The remaining approximately 8 acres will be split from the rest of the Project to preserve the two existing single-family residences as buffers to the properties to the north.

As part of the Endeavour project, Envisage (the developer) would like to work with the County to both expand the parking area and amenity space for the Chuck Huckelberry Loop (the "Loop") adjacent to the Property and reconfigure the entrance to the Project once vehicles turn from Craycroft Road. (See Figures 1 and 2)

An adjoining property, referred to hereafter as "Parcel B", west of Parcel A, is also included in the Specific Plan rezoning application. Parcel B includes four parcels (Pima County Assessor Parcel Numbers 109-26-005L, -005H, -005M, and a portion of -1050) and encompasses

approximately 13.43 acres, is currently within unincorporated Pima County ("County") and is zoned County Suburban Ranch ("SR") and CR-1. Parcel B is proposed as primarily single-family residential development with one 3.51-acre lot (Lot 1 on the Preliminary Development Plan) containing multiple existing single family "casitas" that are rental homes. (See Figure 3)

II. Pima County's Conservation Lands System

This analysis of existing and proposed impacts to the County's Conservation Land System ("CLS") Important Riparian Area (IRA) for both Parcel A and Parcel B will address both on-site (for Parcels A and B separately) and off-site (Pima County staging area) areas. Impacts to CLS Biological Core Management Area (BCM) are discussed in Appendix B.

1. Existing Mapping – All CLS Categories

Approximately 8 acres of Parcel A is mapped CLS and is divided into two categories. Important Riparian Area (IRA) makes up approximately 3.70 acres and Biological Core Management (BCM) makes up approximately 4.38 acres.

Approximately 1.58 acres of Parcel B is mapped CLS and is divided into two categories. Important Riparian Area (IRA) makes up approximately 0.57 acres and Biological Core Management (BCM) makes up approximately 1.00 acres.

The County's existing staging area at Craycroft Road and the Rillito River Park is also designated IRA, with a small area of BCM. Much of this area includes the existing parking lot, pedestrian plaza and buildings. The IRA, up to the mapped BCM area on the east side of the IRA, is included in this analysis because of the proposed off-site impacts for the Project. The impacts come from a new access driveway through the existing staging area. The total area of IRA in the County staging area is 3.6 acres. The CLS area from the County's property (off-site CLS) and impacts to this area are included in mitigation plans for Parcel A because Parcel A requires access though this off-site area. The total amount of mapped IRA on-site and off-site for Parcel A is about 7.3 acres. (See Figures 4 and 5)

(Access to Parcel B comes from an existing roadway to the north of that parcel.)

2. Existing Disturbance - IRA

The areas mapped IRA on-site for Parcel A include previous disturbance from the existing residence. The amount of existing disturbance in the IRA is 2.7 acres or about 73% of the existing mapped IRA. A review of historical aerial photos from 1998 show that this disturbance occurred prior to the adoption of the CLS in 2001 and the Riparian Habitat Ordinance in 1998.

The remaining undisturbed IRA on-site is 0.94 acre.

The amount of existing disturbance in the IRA off-site, on County property is about 1.9 acres or 52% of the existing mapped area.

The remaining undisturbed IRA off-site is 1.66 acres.

The combined area of existing disturbance on-site and off-site is 4.6 acres, or 63% of the total mapped area. (See Figure 6)

The areas mapped IRA on-site for Parcel B include previous disturbance from the existing residence to the west of Parcel B (and owner of Parcel B). The amount of existing disturbance in the IRA is 0.17 acres or about 29% of the existing mapped IRA. A review of historical aerial photos from 1998 show that this disturbance occurred prior to the adoption of the CLS in 2001. (See Figure 7)

3. Proposed Disturbance-IRA

The area mapped as IRA in the southeast corner of Parcel A will be used for the Endeavour Project to provide the mixed-use component of the Endeavour Project, specifically an amenity area serving both river park users and residents of the Project and for Project residential buildings. The expanded public parking area that is in this area will provide a net increase of 30 new parking spaces for use by the public.

Parcel A improvements in the IRA area, both on-site and off-site, will be designed to preserve as many of the existing trees as possible. Some trees on the County property will need to be removed for the new entry drives to the Project and the new parking/staging area. The alignment of these new drives was developed with inventory data on the existing trees, and was established to preserve the larger, healthy trees, while removing trees that are smaller and/or in poorer condition.

The proposed disturbance to the IRA on-site for Parcel A, in excess of the existing disturbance, will be .16 acres. This is 17% of the existing, undisturbed area and about 4% of the total mapped IRA.

The proposed disturbance to the IRA off-site, but included in Parcel A's impacts, in excess of the existing disturbance, will be 0.25 acres. This is 15% of the existing, undisturbed area and about 7% of the total mapped IRA.

The total amount of proposed disturbance to IRA for Parcel A, both on-site and off-site, in excess of existing disturbance, will be .41 acres, or 15% of the undisturbed area and about 6% of the total mapped area of IRA.

2.6 acres of IRA will remain undisturbed on Parcel A between the on-site and off-site areas. Most of this area is along the Rillito River Park and includes existing, large mesquite trees. A small area in the southwest corner of the Project is also IRA and will be preserved. (See Figure 8)

The area mapped IRA in the southern portion of Parcel B will be included in either a Common Area or as part of a new large single family residential lot. No development will occur in the IRA on Parcel B, but revegetation and habitat restoration may occur in this area at the discretion of the property owner.

4. On-site Mitigation- IRA

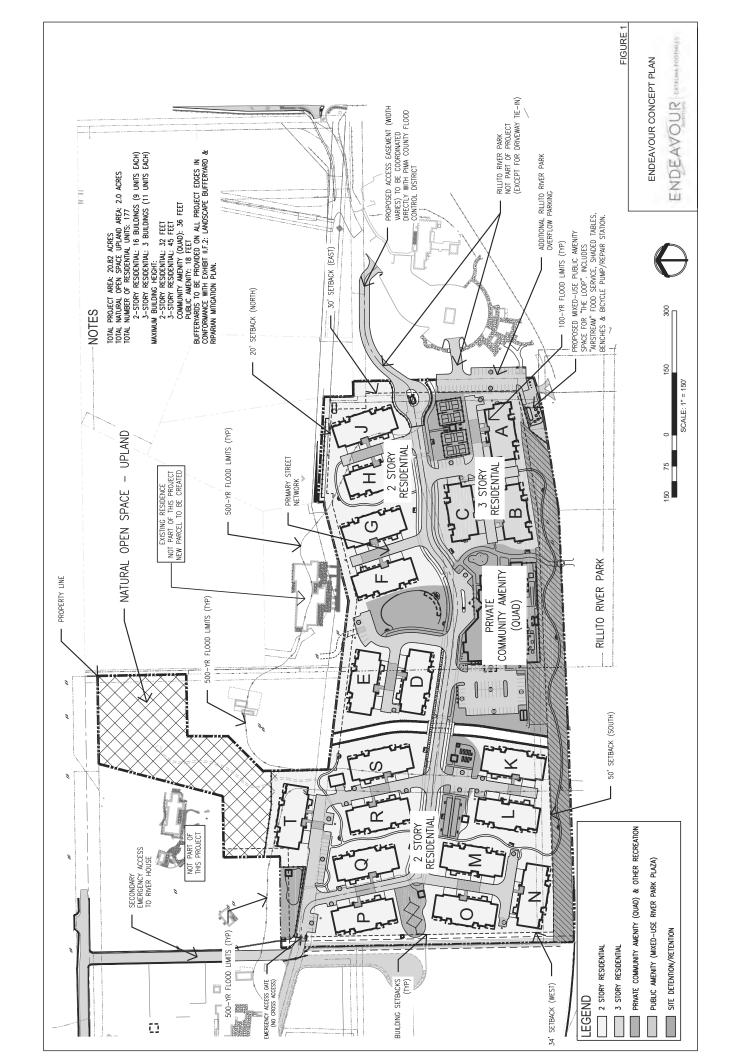
Parcel A, the Endeavour Project, will mitigate for new disturbance to the IRA, both on-site and off-site, through the planting of new riparian trees along the southern edge of the Project, near the eastern end along the boundary of the river park. Other areas of the project will also be revegetated to "riparian" standards, but these areas will be attributed to BCM mitigation.

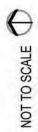
Whether it is attributed to IRA or BCM mitigation, Endeavour will create substantially more riparian habitat than the existing conditions at the time of rezoning.

The total amount of enhanced and/or created riparian habitat on-site for Parcel A will be .82 acres. This is two times the amount of undisturbed IRA that will be disturbed by the Endeavour Project, and exceeds the mitigation ratio of 1.5:1 as included in the Riparian Ordinance.

In addition to the riparian habitat mitigation areas, other areas of the Endeavour Project will be landscaped with trees and shrubs providing additional habitat beyond those areas specifically identified as riparian habitat mitigation.

Parcel B includes two areas that have existing riparian habitat that could be classified as Hydro/Meso Riparian according to the standards of Pima County but are un-mapped and unregulated. These areas will be preserved as part of the site plan for Parcel B. The preservation of these areas which total 1.00 acre are about double the mapped IRA on Parcel B These areas will be attributed to BCM on-site mitigation. (See Figure 9)





ENDEAVOUR CATALINA FOOTHILLS

FIGURE 3 ENDEAVOUR DATALINA FOOTHILL PARCEL B CONCEPT PLAN SINGLE FAMILY LOTS
EXISTING RESIDENTIAL
OPEN SPACE (RIPARIAN AREAS AND COMMON AREAS) LEGEND CR-1 RIVER HOUSE CANYON ESTATES (1-17) (NO CROSS ACCESS) 50 C.A. "A" (PRIVATE STREET) PROPOSED ACCESS GATE CR-1 UNSUBDIVIDED SEQ. 20102100308 (VACANT) LOT 2 0.61 AC LOT 1 0.34 AC 150 LOT 9 2.35 AC LOT 8 0.85 AC 1.04 AC LOT 7 0.82 AC SCALE: 1" = 150' C.A. "B" 0.21 AC 150 75 0 C.A. "C" 0.50AC *Common Area E may be added to Lot 9 at owner's discretion. LOT 6 0.64 AC PROPOSED ACCESS GATE PHASE II EXTENTS SR UNSUBDIVIDED SEQ. 20172780304 RESIDENTIAL SR UNSUBDIVIDED DKT. 6864, PG. 628 RESIDENTIAL CR-1 UNSUBDIVIDED SEC. 2001010785 RESIDENTIAL EXISTING POND MAYBE
RECONFIGURED PER OWNER'S
DISCRETION WITHIN
DESIGNATED COMMON AREA SR ESTATES NO.3 BK. 12, PG.81 RESIDENTIAL

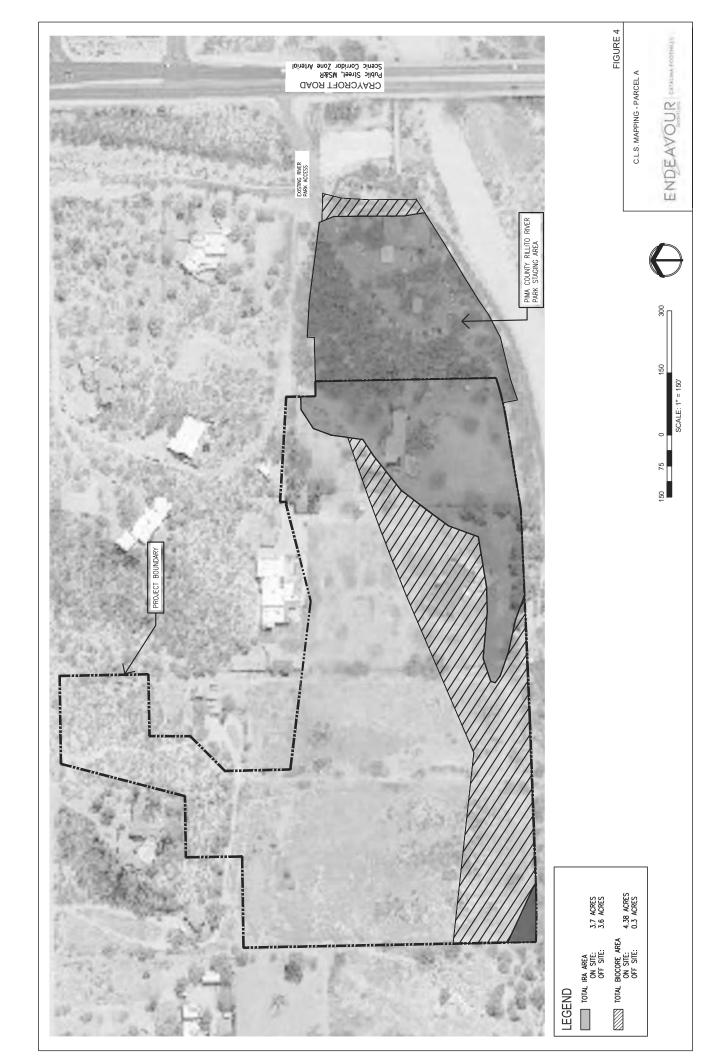
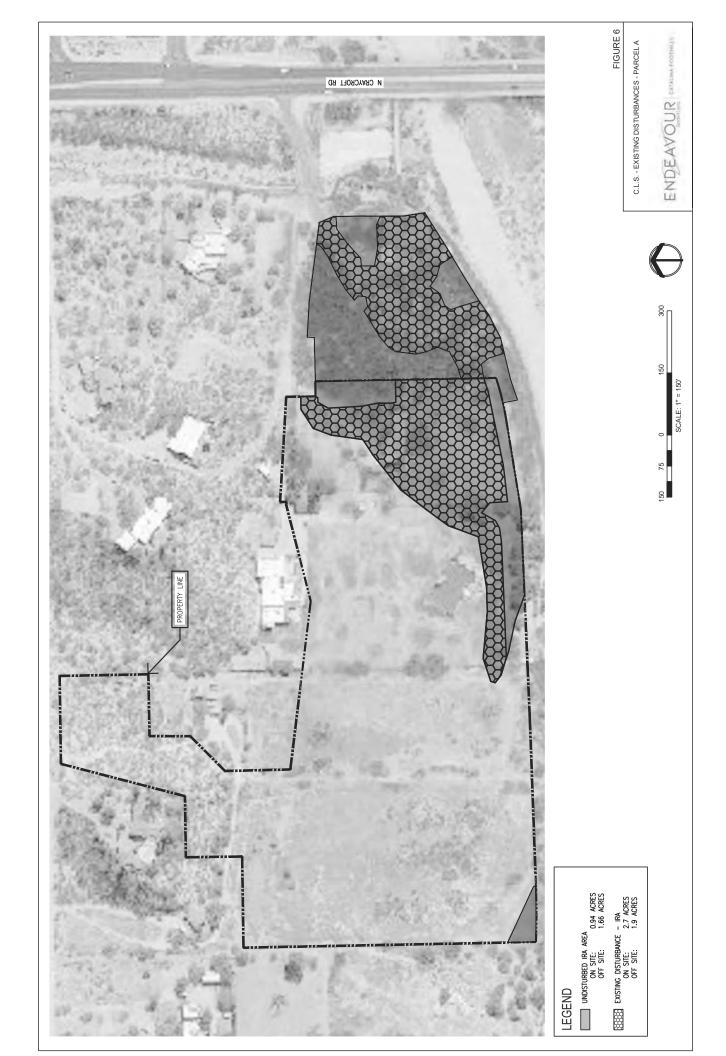


FIGURE 5
C.L.S. MAPPING FOR PARCEL B
ENDEAVOUR CATALINA FOOTBUILD

LEGEND
TOTAL IRA AREA
ON-SITE:
0.57 ACRES
ON-SITE:
0.00 ACRES



C.L.S. WITH EXISTING
DISTURBANCES FOR PARCEL B
ENDEAVOUR PARADIM PORTHUM

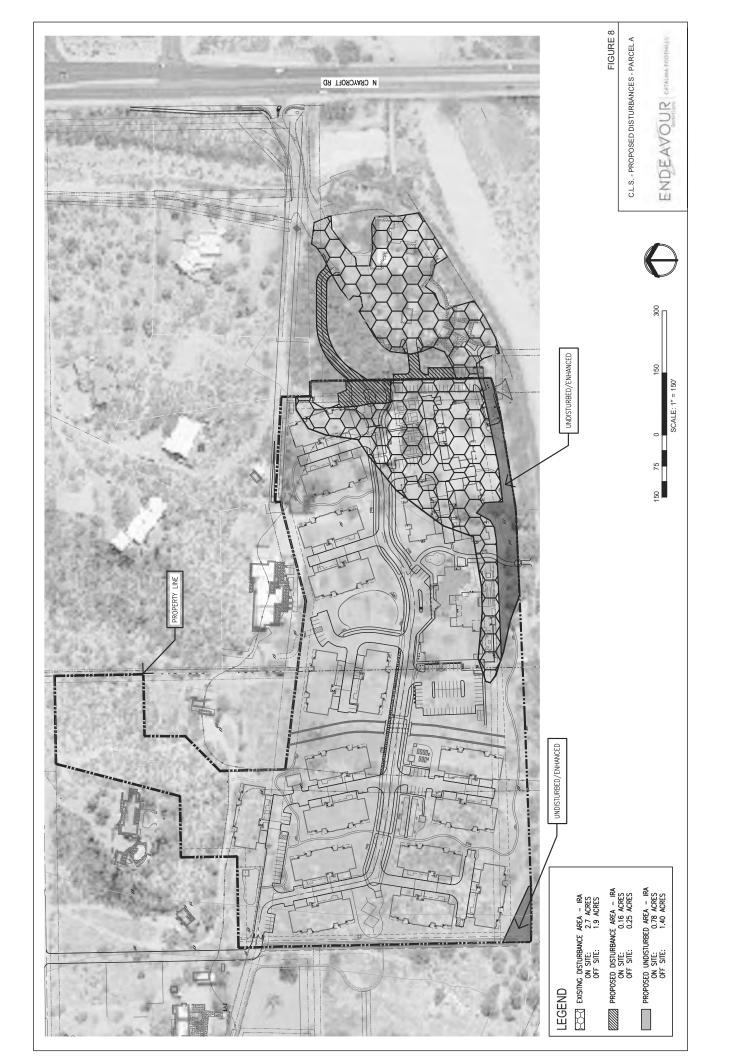
SCALE: 1" = 150'

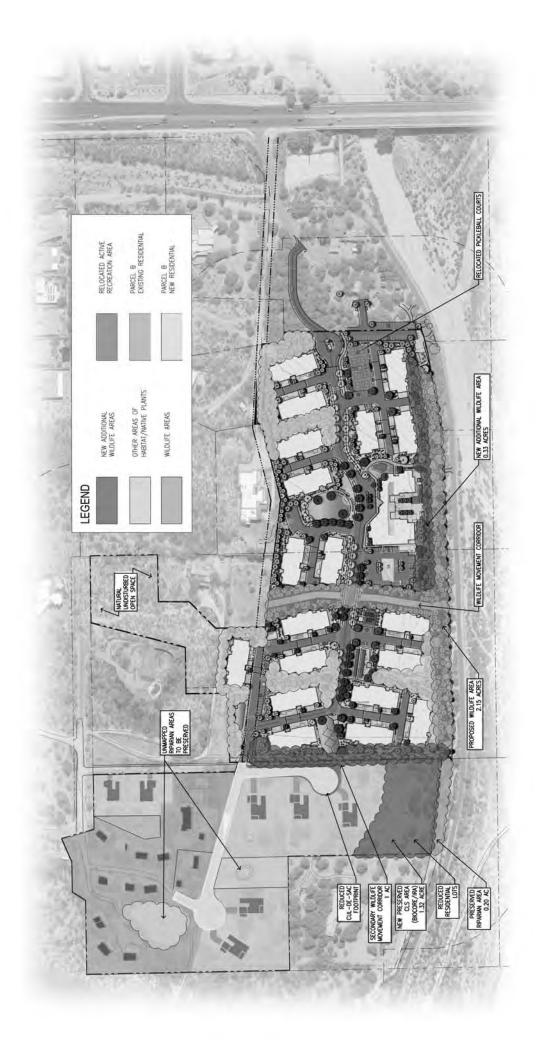
TOTAL IRA AREA

ON-SITE:
0.57 ACRES

TOTAL PREVIOUSLY DISTURBED AREA
ON-SITE:
0.17 ACRES

LECEND TAX ORDER DIAM TO SEE TO SE TO SE





Wildlife Habitat Areas

Figure 9





Appendix B

Endeavour Spirited Living Conservation Land System Biological Core Management Area Analysis



Endeavour Spirited Living Specific Plan Parcels A and B

Conservation Land System Analysis
For Biological Core Management Area (BCMA)
Prepared by
Novak Environmental, Inc.
Karen Cesare, RLA, ASLA

May 25, 2022

I. Introduction

Envisage Living Communities ("Envisage") is proposing to develop Endeavour Spirited Living, which is an active adult independent living project (The Endeavour Project) west of Craycroft Road and south of River Road, referred to hereafter as "Parcel A". Parcel A includes four parcels (Pima County Assessor Parcel Numbers 109-26-003D, -004C, -004D and -003H) and encompasses approximately 28 acres, is currently within unincorporated Pima County ("County") and is zoned County Suburban Ranch ("SR"). Approximately 20 acres of Parcel A will be rezoned to Specific Plan ("SP") to support development of this unique Project. The remaining approximately 8 acres will be split from the rest of the Project to preserve the two existing single-family residences as buffers to the properties to the north.

An adjoining property, referred to hereafter as "Parcel B", west of Parcel A, is also included in the Specific Plan rezoning application. Parcel B includes four parcels (Pima County Assessor Parcel Numbers 109-26-005L, -005H, -005M, and a portion of -1050) and encompasses approximately 13.43 acres, is currently within unincorporated Pima County ("County") and is zoned County Suburban Ranch ("SR") and CR-1. Parcel B is proposed as primarily single-family residential development with one 3.51-acre lot (Lot 1 on the Preliminary Development Plan) containing multiple existing single family "casitas" that are rental homes. (See Figures 1 and 2)

II. Pima County's Conservation Lands System

This analysis of existing and proposed impacts to the County's Conservation Land System ("CLS") Biological Core Management Areas (BCMA) for both Parcel A and Parcel B will address on-site impacts to CLS Biological Core Management Area (BCMA).

1. Existing Mapping – All CLS Categories

Approximately 8 acres of Parcel A is mapped CLS and is divided into two categories. Important Riparian Area (IRA) makes up approximately 3.70 acres and Biological Core Management (BCM) makes up approximately 4.38 acres.

Approximately 1.58 acres of Parcel B is mapped CLS and is divided into two categories. Important Riparian Area (IRA) makes up approximately 0.57 acres and Biological Core Management (BCM) makes up approximately 1.00 acres.

The County's existing staging area at Craycroft Road and the Rillito River Park is also designated IRA, with a small area of BCM. Much of this area includes the existing parking lot, pedestrian plaza and buildings. The IRA, up to the mapped BCM area on the east side of the IRA, is included in this analysis because of the proposed off-site impacts for the Project. The impacts come from a new access driveway through the existing staging area. The total area of IRA in the County staging area is 3.6 acres. The CLS area from the County's property (off-site CLS) and impacts to this area are included in mitigation plans for Parcel A because Parcel A requires access though this off-site area. The total amount of mapped IRA on-site and off-site for Parcel A is about 7.3 acres.

(Access to Parcel B comes from an existing roadway to the north of that parcel.)

2. Existing Disturbance- BCMA

The areas mapped BCMA for Parcel A include previous disturbance from the existing residence. The amount of existing disturbance in the BCMA is 3.70 acres or 100% of the existing mapped BCMA. A review of historical aerial photos from 1998 show that this disturbance occurred prior to the adoption of the CLS in 2001.

The areas mapped BCMA for Parcel B include previous disturbance from the existing residence to the west of Parcel B (and owner of Parcel B). The amount of existing disturbance in the BCMA is 1.00 acres or about 100% of the existing mapped BCMA. A review of historical aerial photos from 1998 show that this disturbance occurred prior to the adoption of the CLS in 2001.

3. Proposed Disturbance- BCMA

Despite the existing disturbed condition of the BCMA, the area is mapped under the CLS, therefore disturbance to the mapped area has been quantified and forms the basis for mitigation under the policies of the CLS. Some of the mapped areas will be revegetated to create areas of habitat. These areas are not counted as impacts to BCMA. Only those areas that are not going to be revegetated, but rather will include constructed elements as part of the project (such as buildings, vehicular areas, and recreational features) are factored in to CLS mitigation requirements.

2.82 acres of BCMA will be impacted on Parcel A.

There is no disturbance to BCMA on Parcel B.

The area mapped BCMA in the southern portion of Parcel B will be part of a new large single family residential lot. No development will occur in the BCMA on Parcel B, but revegetation and habitat restoration may occur in this area at the discretion of the property owner.

4. On-site Mitigation- BCMA

Strict compliance with the CLS polices for BCMA calls for mitigation of disturbed areas at a 4:1 ratio. Four acres of mitigation (either on-site or off-site) is called for every one acre of BCMA disturbed on site.

With 2.82 acres of BCMA disturbed on site, a total of 11.28 acres of mitigation would be needed for strict compliance.

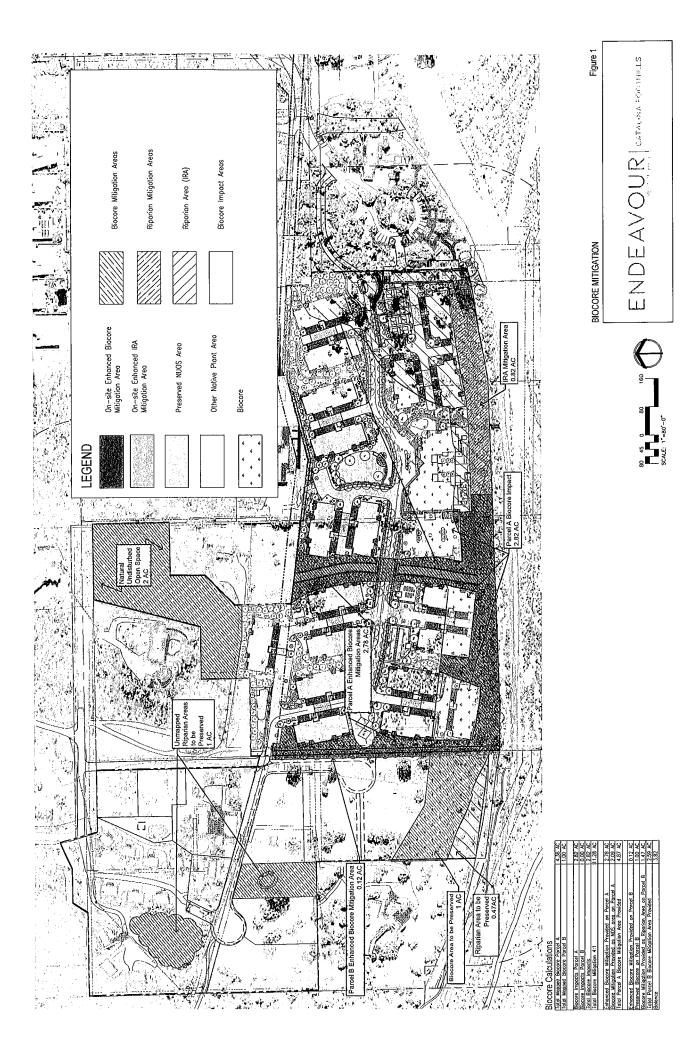
Parcel A, the Endeavour Project, will mitigate for new disturbance to the BCMA on-site through the planting of new riparian vegetation in several areas including along the southern edge of the Project along the existing river park, in the "bioswale" area through the middle of the project and in an area between Parcel A and B. The total amount of on-site revegetated mitigation areas will be 2.90 acres.

Additionally, areas of native vegetation that will be preserved as natural open space will contribute to the on-site BCMA mitigation. These areas include 2.09 acres of Arizona Sonoran Desert Upland including saguaros on the north part of Endeavour and two areas of vegetation totaling 1.0 acre that meet the criteria for "Hydro/Meso" habitat on Parcel B.

A third area of CLS which includes the entire area of the mapped CLS, both IRA and BCM on Parcel B is also included in the on-site mitigation calculations. This area is 1.47 acres.

The total amount of on-site BCMA mitigation area for the project (both Parcels A and B) is 7.46 acres.

While this is 3.82 acres shy of full CLS compliance, there are other mitigation factors that are "qualitative" rather than "quantitative". These include site plan changes made since the initial submittal to create or enhance wildlife habitat elements on site. Many of these are included as conditions to the Specific Plan. Additionally, certain recreational features, specifically the pickleball courts, were relocated away from wildlife movement corridors to reduce conflicts between active uses and wildlife areas. When taken as a whole, and considering the existing conditions of the site, the project will include many specific elements that meet the intent of the CLS to support wildlife habitat.



Appendix C

Endeavour Spirited Living Traffic Impact Study

Endeavor Spirited Living

Traffic Impact Study

Prepared for submittal to:

City of Tucson, AZ Pima County



M Esparza Engineering, LLC 2934 W. Salvia Drive Tucson, AZ 85745

December 17, 2020 Updated June 4, 2021 Updated November 16, 2021 Updated June 6, 2022

Endeavor Spirited Living Traffic Impact Study

Prepared for Submittal to:

City of Tucson, Arizona Pima County

Prepared by:

M Esparza Engineering
2934 W. Salvia Drive
Tucson, AZ 85745

(520) 207-3358 Project No. 2020.03 MARCOS U ESPARZA VIEW Signed LA VIEW

Marcos Esparza, P.E., Principal

December 17, 2020 Updated June 4, 2021 Updated November 16, 2021 Updated June 6, 2022

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1. Introduction and Summary of Key Findings

Project Overview

This report analyzes traffic impacts from a proposed 34-acre project located west of Craycroft and south of River Road (the "Project"). Parcel A is 20 acres of the Project to be developed as an innovative, highly-amenitized active adult independent living community called Endeavor Spirited Living. The Project location is shown in Exhibit 1. Parcel A will include 177 residential units with innovative supporting services and amenities. The site plan for this main project is shown in Exhibit 2 and in the report appendix. Supporting services as shown on the site plan include food service options, recreation areas (indoor and outdoor), an administration area and staff areas. The proposed development is near the Pima County ("County") Rillito River Linear Park. Primary access to Parcel A is proposed from Craycroft Road and is shared with the County park access road and referred to herein as the "Main Access Road". An emergency-only secondary access is also discussed in this report.

Parcel B is located on the west side of Parcel A and proposes an additional seven (7) homes in a nine-lot single-family residential development. (There are currently 12 rental residences already located on the Parcel B.) Access to the lots will be via an existing north/south road along the eastern edge of Parcel B. This north/south road will intersect with River House Road which has an existing access to Camino Blanco to the north and ultimately River Road. There is an emergency-only access gate that separates Parcels A and B, so there will be no sharing of the roadways between the two projects.

A meeting was held with City of Tucson Department of Transportation and Mobility (COTDTM) staff in March 2020 to discuss Parcel A. The purpose of the meeting was to discuss potential improvements on the Main Access Road and at its intersection with Craycroft Road to enhance safe conditions within the Project study area. COTDTM staff requested that we explore a similar type of residential community to our project with comparable land uses and area context within the City of Tucson. Our research did not identify an existing senior living community that was comparable both in type and area; therefore "Congregate Care Facility" has been used for Parcel A in this analysis.

Purpose of Report

This analysis addresses impacts from the Project on the surrounding roadways and intersections. The Project area is within the County's jurisdiction, but the Main Access Road for Parcel A is within the City of Tucson ("City").

The report has thus been prepared in accordance with the requirements within both the *Transportation Access Management Guidelines for the City of Tucson* for a Category I traffic impact analysis (TIA) and Pima County's *Subdivision and Development Street Standards*. The level of TIA under both jurisdictions' requirements is done for a single-phase development with fewer than 500 peak hour trips. The study analyzes the impacts in the opening year, 2023 and the study area includes the site access driveways and all adjacent signalized and/or major unsignalized street intersections. The roadways and intersections analyzed in this report include the following:

Roadways

Craycroft Road River Road

Intersections

Craycroft Road/River Road (signalized)
Craycroft Road/Gregory School (signalized)
River Road/Camino Blanco (unsignalized)
Craycroft Road/Main Access Road (unsignalized)



The purpose of this report, in addition to analyzing the study area roadway system and the impacts of the Project, is to make careful recommendations for traffic and roadway improvements. Because the Project will serve an elderly population, the recommendations in this report are provided to optimize the safety and ease of access for the Parcel A residents and staff and the Parcel B residents as well as for those drivers that currently use the Main Access Road and Camino Blanco.

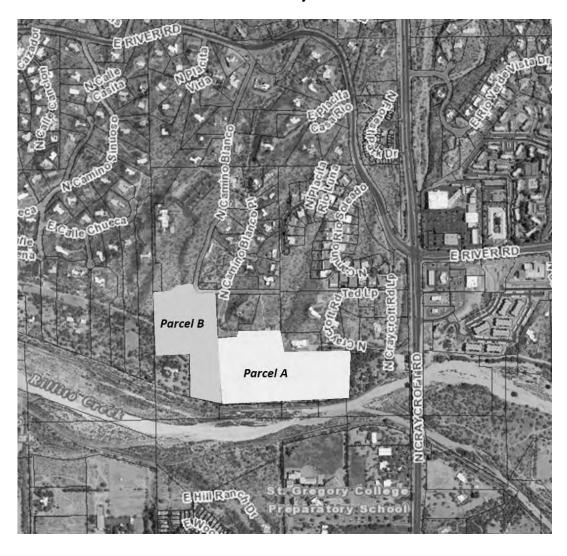
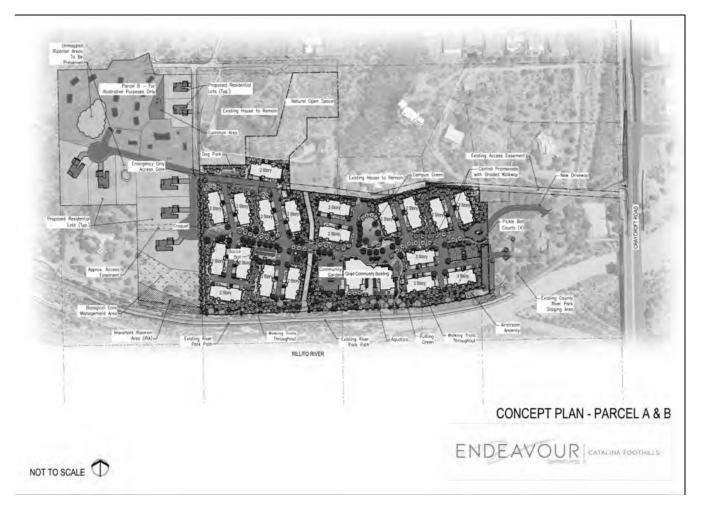


Exhibit 1 Project Location

Exhibit 2 Conceptual Site Plan – Parcels A & B



Methodology

This analysis uses recorded weekday AM and PM peak hour volumes from 2018 and 2020. We have applied a 2% growth factor to estimate year 2022¹ and 2023 conditions. We analyzed both "no Project" and "with Project" scenarios. For the "no Project" scenarios, we increased the recorded traffic volumes by 2% per year and analyzed conditions for the year 2023.

In order to estimate the trips for Parcel A, we reviewed the land use types in the ITE Trip Generation Manual to determine the closest land use to this project. The description of the land use "Congregate Care Facility" (ITE Land Use Code 253) was determined by the Project team to be the best representation of the type of facility this Project will be. For Parcel B, we applied the trip rates for the land use "Single Family Detached Housing" (ITE Land Use Code 210). The average trip generation rates for the daily, AM peak hour and PM peak hour time frames were applied to estimate the Project site trips. Based on comments from Pima County DOT, we have also provided the estimated site trips based

¹ The preparation of this project began in 2020 and existing older traffic volumes were "normalized" to estimate 2020 traffic volumes. Because of the impact on traffic volumes and patterns because of the COVID-19 pandemic, we believe that traffic volumes did not increase in the project area between 2020 and 2021. Therefore the 2% increase per year to estimate the original 2020 volumes assumes that the 2021 volumes would be the same as the 2020 volumes. The original buildout year was 2022, but has been updated to 2023, and the estimated 2022 volumes have been applied to the new buildout 2023 volumes for the same reason. To be clear, the 2023 volumes were estimated by applying a 2% rate per year for two years from the 2021 volumes.



on the trip rates for the AM and PM Peak Generator time periods and updated the trip generation estimates to reflect changes in the trip generation rates in the recently updated 11th Edition of the Institute of Transportation Engineers' *Trip Generation Manual*.

The estimated site trips from the Project were then added to future background volumes to provide an estimate of total traffic for the buildout year. To be conservative, we applied the site trips associated with the AM and PM Peak Generator times during the AM and PM peak hours. We then analyzed the study area intersections using the Synchro software program to determine what impacts the Project would have and to recommend mitigation to bring the intersections to acceptable levels of service. The analyses were conducted for the Project year, with and without site trips, to clearly demonstrate the impacts from the Project.

Summary of Key Findings

- The proposed Parcel A project will generate about 391 trips during the average weekday, with about 14 during the morning commuter peak hour and 32 during the afternoon/evening commuter peak hour². During the times when Parcel A will generate the highest morning and afternoon/evening hourly trips, the project will generate about 34 AM trips and 41 PM trips.
- The proposed Parcel B project will generate about 66 new trips during the average weekday, with about 5 during the morning peak hour and 7 during the afternoon/evening peak hour.
- Current conditions:
 - o The intersection of River/Craycroft currently operates at LOS E or F during the morning and afternoon peak hours.
 - The eastbound left turn movement at the Main Access Road intersection on Craycroft Road operates at LOS F during the morning peak hours and delays will increase without and with the Project through 2023. Delays are common for driveways or minor streets that enter major streets during peak commute hours.
 - The intersection of Gregory School/Craycroft currently operates at LOS C during the peak hours and will continue to operate at LOS C through 2023 with the Project.
 - o The northbound left turn movement at the River Road/Camino Blanco intersection operates at LOS E during the morning peak hour and will continue to operate at LOS E/F through 2023.
 - o The Main Access Road on Craycroft Road exists and meets City of Tucson standards for driveway spacing and corner clearance.
- Adding the Project site trips to the future years' background traffic volumes will increase
 delays even more at the intersections, although the proportional increases are minor.

Parcel A recommended mitigation:

- Specific Project related intersection mitigation recommendations includes:
 - Re-striping the two-way left turn lane on the northbound Craycroft Road approach to the Craycroft Road/Main Access Road intersection to delineate a 150-foot left turn lane.

² Commuter peak hours are typically the highest volume one hour period in the 7-9 AM morning time frame and the highest volume one hour period in the 4-6 PM time frame.



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- o Provide a warranted southbound right turn lane on Craycroft Road for turns into the Main Access Road. This improvement should only be considered if the sight distance for drivers entering Craycroft Road from the Main Access Road is not reduced to an unacceptable distance because of the improvement.
- o Reconstructing the Main Access Road to accommodate the lane configuration near Craycroft as shown in the site plan. Outbound traffic is restricted to right-out turning movements.
- Providing stop control inside the reconstructed Main Access Road at the internal intersecting roads between the Rillito River Park trail head and the new Project/residences to the north.
- Drivers turning left out of the Main Access Road today experience delays representative of LOS E or LOS F conditions. Elderly (or any) drivers wishing to head north of Craycroft Road from the Main Access Road would likely opt to turn right from the Main Access Road onto Craycroft and seek a downstream opportunity to turn around and head north. It is recommended that outbound movements be restricted to right turns out only to eliminate the potential for eastbound to northbound left turn crashes. The following improvement at the downstream intersection of Craycroft Road/Gregory School is recommended.
- To assist exiting drivers from the Main Access Road onto Craycroft Road, a new raised median and delineated U-turn lane is recommended at the Craycroft Road/Gregory School access for the north leg of the intersection. The purpose of this is to provide a southbound U-turn lane at the intersection for use by drivers whose destinations are north of the Project access driveway and who would be restricted to the recommended right-out only movement at the Main Access Road intersection with Craycroft Road. The north leg of the Craycroft Road/Gregory School intersection would be reconstructed and restriped to provide a 150-foot left turn lane with a raised median separating northbound and southbound traffic. If this recommendation is approved, a left turn phase warrant analysis should be conducted for the southbound left turn lane.

Parcel B recommended mitigation:

 Parcel B trips will be only via Camino Blanco to River Road. Access from Parcel A to Camino Blanco will be only for emergencies through a gated access on the west side of the project. No additional mitigation is recommended on River House Road, Camino Blanco or River Road.

2. Proposed Development

Site Location

The Project is located near the southwest corner of the intersection of Craycroft Road and River Road in Pima County. It is in an area surrounded by commercial, institutional, and single-family residential uses.

Proposed Development and Access

Parcel A of the Project includes 177 senior living residential units sited on about 20 acres of a 28-acre site. Parcel B includes 9 single family residential lots with 7 new homes proposed. The site plan is shown in Exhibit 2.

The following describes important features of the Parcel A Project:

- Parcel A will be age-restricted to 55 years and older. However, the targeted demographic is over 70 years old.
- Parcel A will support an independent living lifestyle for the targeted demographic of active adult seniors in a small neighborhood setting.
- Parcel A differs from other senior residential developments in several ways. One difference is
 that it will not be a licensed continuing care or assisted living community and will not provide
 "on-property" health care or medical treatment to its residents. Rather it will emphasize the
 proactive health and wellness of its residents.
- Parcel A will include innovative amenities on-site including a central gathering space featuring dining options (formal dining, sports bar dining, coffee bar) with staff providing full dining service as well as take out services for residents and their guests. The on-site dining amenities generally differ from traditional senior living or assisted living developments that contract out the dining services.
- Parcel A will have three to four activity specialists (therapists and exercise trainers) as regular staff instead of contracting with outside providers.
- Staffing shifts will be "flexed" to be outside of peak commute hours.

There is one access location shown on the plan, on Craycroft Road, via an existing access (the "Main Access Road") to a County park and residential lots. A secondary emergency-only gated access via a new connection from the Project to River Road via Camino Blanco is discussed in this report.

The existing Main Access Road for Parcel A is stop-sign controlled at its intersection with Craycroft Road and has one entering lane and one exiting lane. It is just north of a bridge over the Rillito River. The Main Access Road is narrow and provides current access to the Pima County park driveway and trailhead and a few residential lots. The Main Access Road would need to be widened to provide two-way paved access to Parcel A. This will require the removal of existing vegetation along the existing road. Separate left and right turn lanes should be provided on the Main Access Road to minimize delays for drivers entering Craycroft Road. Stop sign control should be provided on the intersecting roadways that provide access to the residential lots to the north and the Pima County park driveway and trailhead.

Trips to and from Parcel B will be via the existing River House Road to Camino Blanco. Secondary emergency-only access for Parcel A will be provided at a gated access to the Parcel B north/south roadway. Further discussions with the jurisdictions will determine the level of improvements, if any, need to be made to this access.

Development Phasing and Timing

For the purposes of this study, both parcels are expected to open in 2023.

Study Area

The study area includes all major roadways, and major intersections in the vicinity of the Project. This includes Craycroft Road and the existing intersections of Craycroft Road/River Road, Craycroft/Gregory School Road, Craycroft/Main Access Road and River Road/Camino Blanco.

Area of Significant Traffic Impact

Significant impact from this Project will be on all roadways and intersections in the vicinity of the Project.

Influence Area

Parcel A will draw staff and visitors from the general Tucson area and possibly beyond. It will be a specific destination for employees served by this land use. Parcel B will serve residents living on Parcel B.

Site Accessibility

Parcel A will be served by Craycroft Road. Parcel B will be accessed to the north from River House Road to Camino Blanco and ultimately to River Road. On the City of Tucson Major Streets and Routes Map, Craycroft Road is classified as an arterial street between Golf Links Road to north of Fort Lowell Road where it changes classification to a Scenic Arterial Street. Craycroft Road is a four-lane roadway in the vicinity of the Project with a two-way left turn lane. The area is easily accessible via Tucson's and Pima County's arterial and collector roadway system.

River Road is classified as a Low Volume Arterial on Pima County's Major Streets Plan and a Scenic Major Route on Pima County's Scenic Routes Plan. It is a two-lane roadway that is also easily accessible via Tucson's and Pima County's arterial and collector roadway system.

Camino Blanco is a paved two-lane local roadway maintained by Pima County.

Future Roadway Improvements

There are no funded roadway improvement Projects near the Project currently.

3. Analysis of Existing Conditions

Physical Characteristics

This section provides a description of the roadways that provide access to the Project. Exhibit 3 provides a physical inventory of the study area roadways and Exhibit 4 contains ground photographs showing the roadways as they exist today.

Existing Roads Adjacent to the Project

<u>Craycroft Road</u> is a north/south arterial road with a five-lane cross-section near the Project. There are two travel lanes in each direction with a two-way left turn lane. There is a bike lane on each side of the roadway. Curbs and sidewalk and walls exist along both sides of the road. The posted speed limit is 45 mph north of Gregory School Road and 40 mph south of Gregory School Road.

<u>River Road</u> is an east/west arterial road approximately 870 feet north of the Project access on Craycroft Road. It is a two-lane roadway with a posted speed limit of 35 mph. There are sidewalks, curb and gutter and bike lanes on the south side of the road near Craycroft Road.

<u>Main Access Road</u> is a private road providing access to a trailhead of the Rillito River Park and to residential lots. The existing road is narrow and would need to be widened to a two-lane cross section.

<u>Camino Blanco</u> is a north/south local road providing access to residential and institutional uses south of River Road. It continues south for about ¾ mile to its intersection with the unpaved River House Road. There are no curbs, sidewalks, or bike routes.

Exhibit 3 Roadway Inventory

Roadway Segment	No. Lanes	Median	Bike Facility	Ped Facility	Speed Limit
Roduway Segment	INO. Lailes	Median	racility	•	LIIIII
Craycroft Road, North of	4	Raised	Striped	Sidewalks on both	45
River Road			Lane	sides	
Craycroft Road, South of			Striped	Sidewalks	
River Road	4	TWLTL	Lane	on both	45/40
Niver Noau			Larie	sides	
River Road, East of			Striped	Sidewalk on	
Craycroft Road	2	TWLTL	Lane	the south	35
Craycroft Road		Lane		side	
River Road, West of	2	TWLTL	Striped	Sidewalk on	35
Craycroft Road	2	IVVLIL	Lane	south side	33
Main Access Road	Not defined, but narrow	No	No	No	N/A
Camino Blanco	2	No	No	No	25

Exhibit 4 Ground Photographs



Main Access Road – looking west



Looking east toward Craycroft Road

Exhibit 4 (cont.) Ground Photographs



Looking south on Craycroft Road at Main Access Road



Looking north on Craycroft Road from Main Access Road

Traffic Volumes and Level of Service

Level of service (LOS) is a qualitative description of how well a roadway operates under prevailing traffic conditions. A grading system of A through F, similar to academic grades, is utilized. LOS A is free-flowing traffic, whereas LOS F is forced flow and extreme congestion. Level of service D is the assumed performance standard in the Project area during the peak periods. Traffic volumes, roadway capacity and planning level LOS of the surrounding roadways are provided in Exhibit 5. Capacities are taken from the FDOT generalized LOS Tables³. Average daily volumes were recorded between 2018 and 2020⁴ as shown in the table.

The table shows that most roadway segments are operating over the LOS D capacity of the segment. based on the FDOT LOS guidelines.

Exhibit 5 Current Segment Performance

Roadway Segment	Year	LOS D Capacity	Recorded ADT
Craycroft Road, North of River Road	2019	35,820	30,151
Craycroft Road, North of Project Driveway	2020	35,820	37,603
Craycroft Road, South of Project Driveway	2020	35,820	37,545
River Road, East of Craycroft Road	2019	13,320	16,017
River Road, West of Craycroft Road	2019	13,320	13,920
Camino Blanco, South of River Road	2018	n/a	220 (estimated)

Existing Intersections

The existing Project area signalized intersections at Craycroft Road/River Road and Craycroft Road/Gregory School Road experience heavy commuter traffic during the morning and afternoon peak hours.

River Road/Craycroft Road has dual left turn lanes on the westbound approach and one exclusive left turn lane on the other approaches. Each approach has an exclusive right turn lane. Each approach has a protected leading left turn phase.

⁴ The last traffic volumes were collected in February 2020, before COVID-19 shut down schools and impacted travel, therefore these traffic volumes are considered "typical".



³ Florida Department of Transportation Level of Service Tables,2012

Craycroft Road/Gregory School is a three-leg intersection. The Gregory School approach has a one exclusive left turn lane and an exclusive right turn lane. There is a leading left turn phase on the northbound approach.

The Craycroft/Main Access Road is a three-leg intersection and is stop-sign controlled on the minor approach.

River Road/Camino Blanco is a three-leg intersection and is stop-sign controlled on the minor (Camino Blanco) approach.

Intersection Performance

The most recently recorded intersection peak hour volumes used in this analysis are provided in the Appendix of this report. These volumes were collected in 2018 or 2020 and were provided by the Pima Association of Governments Transportation Data Management System webpage (Craycroft Road/Gregory School, data collected in 2018), or were collected by Field Data Services of Arizona (River Road/Craycroft Road and Craycroft Road/Project Access, data collected for both in 2020 and River Road/Camino Blanco, data collected in 2018). To estimate 2020 volumes for the 2018 volumes, an annual growth rate of 2%/year was applied to the recorded volumes. Because of COVID, we have applied the 2020 volumes as being more typical than volumes in 2022. These volumes are shown in Exhibit 6.

We analyzed the intersections using the software program Synchro 10. This program provides delay and level of service based on the Highway Capacity Manual methodology for intersections. As shown in Exhibit 7, the operational analysis of the intersections indicates that the signalized intersection of River Road/Craycroft Road currently has movements that operate at LOS E or F during the peak hours. The signalized intersection of Craycroft Road/Gregory School operates at LOS D or better during the peak hours. At the Craycroft Road/Main Access Road intersection, the eastbound approach to Craycroft experiences LOS E during the AM peak hour. The northbound left turn lane movement at the River Road/Camino Blanco intersection operates at LOS E during the AM peak hour.

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Exhibit 6 Current Peak Hour Volumes

Exhibit 7 Current Intersections Performance

River/Craycroft

	E	kisting	g (2020)	
	AM		PM	
	Delay		Delay	
	(sec/veh)	LOS	(sec/veh)	LOS
Eastbound				
Left	38.4	D	32.4	С
Through	43.8	D	48.2	D
Right	55.6	Ε	33.1	С
Approach	47.2	D	43.8	D
Westbound				
Left	50.9	D	51.8	D
Through	40.4	D	39.1	D
Right	37.9	D	40.4	D
Approach	45.3	D	44.5	D
Northbound				
Left	86.5	F	35.6	D
Through	38.8	D	100.8	F
Right	18.9	В	29.1	C E
Approach	39.9	D	75.7	E
Southbound				
Left	46	D	83.2	F
Through	162.9	F	51.3	D
Right	22.4	С	22.9	С
Approach	143.8	F	56.8	E
Intersection	81.5	F	60.2	Ε

Craycroft/Gregory School

	E	kistin	g (2020)	
	AM		PM	
	Delay		Delay	
	(sec/veh)	LOS	(sec/veh)	LOS
Eastbound				
Left	41.7	D	21.6	С
Right	48.1	D	21.6	С
Approach	45.6	D	21.6	С
Northbound				
Left	19.6	В	10.6	В
Through	2.2	Α	20.9	С
Approach	3	Α	20.7	С
Southbound				
Through	15.5	В	18.7	В
Right	15.9	В	18.5	В
Approach	15.7	В	18.6	В
Intersection	12.2	В	20.0	С

Craycroft/Project Driveway

	E	kistin	g (2020)	
	AM		PM	
	Delay		Delay	
	(sec/veh)	LOS	(sec/veh)	LOS
Eastbound				
Left	125.7	F	33.8	D
Right	30.1	D	13.5	В
Northbound				
Left	26	D	11.4	В

River Road/Camino Blanco

	E	xistin	g (2020)	
	AM		PM	
	Delay		Delay (sec/veh)	
	(sec/veh)	LOS	(sec/veh)	LOS
Westbound				
Left	8.6	A	9.9	Α
Northbound				
Left	37.7	Е	32.3	D
Right	12.1	В	16.4	С

Traffic Safety

Vehicle crash data at the nearest intersection, River Road/Craycroft Road, and the roadway segments on River Road and Craycroft Road in the vicinity of the Project were provided by the Arizona Department of Transportation for the period of January 1, 2016, thru December 31, 2018. Exhibits 8a and 8b summarize the number of crashes by crash type, injury severity and crash rate. There were no intersection crashes recorded during this period for the intersections of Craycroft Road/Main Access Road or Craycroft Road/Gregory School.

The summary shows that the most common crash types at River/Craycroft were Rear End (9), Left Turn (10) crashes. The three-year crash rate was near 0.50 crashes per million entering vehicles (MEV) with no fatal crashes during the three-year period.

On the roadway segments, most of the non-intersection related crashes were single vehicle (5) and sideswipes (6). There were an equal number of crashes (6) on the River Road segment east of Craycroft Road as on the Craycroft Road segment south of River Road during the three-year period. There were ten property damage only crashes and seven crashes with injury. The segment with the highest crash rate (0.68 crashes per million vehicle miles) was the River Road segment east of Craycroft Road.

Exhibit 8a Crash Rates: Intersection Related

River/Craycroft

Crash Type	2016	2017	2018	Total	%
Single Vehicle (1)		1		1	4%
Angle (2)	1	1		2	7%
Left Turn (3)	1	4	5	10	37%
Rear End (4)	1	4	4	9	33%
Head On (5)		1		1	4%
Side Swipe (6 7)	1	2		3	11%
Other		1		1	4%
Total	4	14	9	27	
Crash Rate (per MVE)	0.22	0.79	0.50	0.50	

Severity				Total	%
Bodily Injury	3	3	4	10	37%
Property Damage	1	11	5	17	63%

Note: MVE = Million Vehicles Entering the intersection

Entering Vols River/Craycroft

48,846 Based on PAG Data

Source of crash data: ADOT; Source of volumes: PAG.

Exhibit 8b Crash Rates: Non-Intersection Related

	2016	16	2017			2018			3 Year Total		
Crash Type	Craycroft Road, South of River	River Road, West of Craycroft	Craycroft Road, South of River	River Road, West of Craycroft	Craycroft Road, South of River	River Road, Road, East of West of Craycroft	River Road, West of Craycroft	Craycroft Road, South of River	River Road, East of Craycroft	River Road, River Road, East of West of Craycroft Craycroft	%
Single Vehicle	2		1	1		-		3	ı	1	79%
Rear End	_			_	_	-		2	1	-	24%
Sideswipe	_			_		ო	_	_	က	7	35%
Other		_						0	0	-	%9
Angle						-		0	1	0	%9
Total	4	-	-	က	-	9	-	9	9	2	
Crash Rate (per MVM)	0.58	0.34	0.34	1.03	0.15	2.05	0.39	0.29	0.68	99.0	
Severity											%
Fatal	0	0	0	0	0	0	0	0	0	0	%0
Bodily Injury	2	_	_		0	2	_	က	2	2	41%
Property Damage	7	0		က	_	4		က	4	က	29%
Total	4	1	1	3	1	9	1	9	9	5	

Note: MVM = Million Vehicle-Miles

Volumes
Graycroft, South of River 37,603 Based on PAG Data
River, West of Craycroft 13,920
River, East of Craycroft 16,017

Source of Crash Data: ADOT; Source of Volumes: PAG.

4. Projected Traffic

Site Traffic Forecasting

The future traffic from Parcel A is estimated using the trip rates contained in the Institute of Traffic Engineers' *Trip Generation, 11th Edition* for land use category 253– Congregate Care Facility and is based on the number of planned residential units (177) of the proposed Project. The project team reviewed the descriptions of residential land use types in the ITE Trip Generation Manual to determine the closest land use to this project. The description of the land use "Congregate Care Facility" was determined by the Project team to be the best representation of the type of facility this Project will be. Pima County staff also approved the use of this land use for the purpose of this analysis early in the development of this project. The average trip generation rates for the daily, commuter AM peak hour and commuter PM peak hour time frames for this land use were applied to estimate the Project site trips. Based on comments from Pima County DOT, we have also provided the estimated site trips based on the trip rates for the AM and PM Peak Generator time periods.

The future traffic from Parcel B is estimated using the trip rates for land use category 210–Single-Family Detached Housing and is based on the number of new dwelling units (7) of the proposed Parcel B area.

Trip generation is the mathematical product of land use intensity (building square footage, number of units, etc.) and the trip generation rate. The result is the total number of one-way trips expected to be generated by the Project. These trips represent the number of vehicles estimated to enter and leave the Project. All the estimates are based on average trip rates per residential unit.

Trip Generation

Exhibits 9a and 9b provide the ITE <u>trip rates</u> and resulting <u>trip generation</u> for the proposed uses during the average weekday. These volumes represent the total number of vehicle trips generated by the Project parcels at the driveways.



Exhibit 9a Trip Generation – Parcel A

					Trip	Gener	ation Ra	ites	
		No.	ITE	Weekd	ay AM	Weeko	lay PM	Avg W	eekday'
Land Use	Unit	Units	Categ.	In	Out	In	Out	In	Out
Congregate Care Facility	Units	177	253	0.08		0.18 2.21		.21	
				58%	42%	49%	51%	50%	50%

					•	Trip Ger	neration	1	
		No.	ITE	Week	day AM	Weeko	lay PM	Avg W	'eekday
Land Use	Unit	Units	Categ.	In	Out	ln	Out	In	Out
Congregate Care Facility	1000 SF	177	253	,	14	32 391		91	
				8	6	16	16	196	196

Note: AM, PM Rates based on Peak Hour of Adjacent Street Traffic (7-9 AM; 4-6 PM)

					Trip	Gener	ation Ra	tes			
		No.	ITE	Weekday AM Weekday PM Avg Week			/eekday				
Land Use	Unit	Units	Categ.	ln	Out	In	Out	In	Out		
Congregate Care Facility	Units	177	253	0.19		0.:	.23 2.21		.21		
				56%	44%	54%	46%	50%	50%		

					•	Trip Generation				
		No.	ITE	Weekday AM Weekday PM Avg We		'eekday				
Land Use	Unit	Units	Categ.	In	Out	ln	Out	In	Out	
Congregate Care Facility	1000 SF	177	253	34		4	1	391		
				19	15	22	19	196	196	

Note: AM, PM Rates based on Peak Hour of Generator

Exhibit 9b Trip Generation – Parcel B

		No.	ITE	Weekday AM		Weekday AM Weekday PM		Avg Weekday		
Land Use	Unit	Units	Categ.	In	Out	In	Out	ln	Out	
Single Family Detached Unit	Units	7	210	0.7		0.9	94	9.	9.43	
				26%	74%	63%	37%	50%	50%	

						Trip Ge	neration)	
		No.	ITE	Weekday AM Weekday PM Avg W			Veekday		
Land Use	Unit	Units	Categ.	In	Out	In	Out	In	Out
Single Family Detached Unit	1000 SF	7	210	5			7	66	
				1	4	4	2	33	33

Note: AM, PM Rates based on Peak Hour of Adjacent Street Traffic (7-9 AM; 4-6 PM)

Trip Deductions

No pass-by trip reductions were applied to this land use category. Pass-by trip reductions are typically applied to shopping centers, restaurants, and convenience stores.

Trip Distribution

Trips generated for both Project parcels have been distributed to the surrounding roadway network and the Project driveways. Parcel B traffic will be completely distributed to River Road via Camino Blanco, and Parcel A traffic will be completely distributed via the Craycroft access. The percentage site distribution for each scenario is shown in Exhibit 10.

Site Traffic Assignment

Using the distribution of total trips shown in Exhibit 10, the site trips are assigned to the roadways and the intersections and driveway. The resulting peak hour assignment at buildout of the Project are illustrated in Exhibit 11.

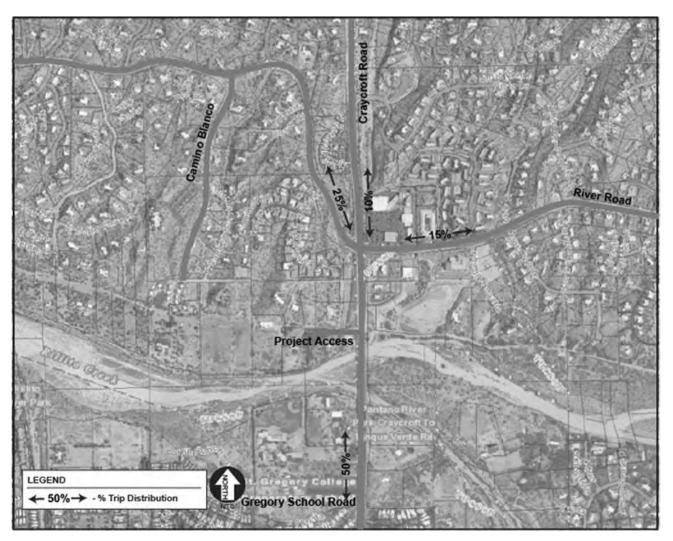


Exhibit 10 Site Traffic Distribution

Note: Distribution shown for Parcel A Trips. Distribution of Parcel B Trips will be 100% via Camino Blanco and distributed equally to the east and west on River Road.

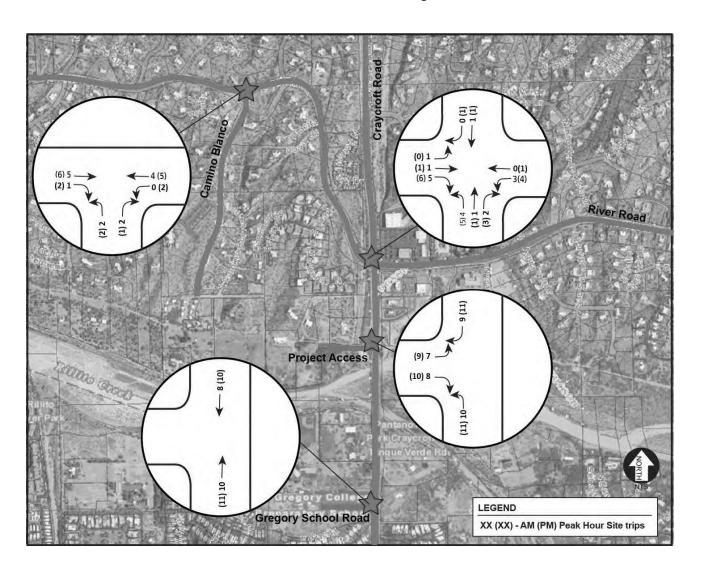
Non-Site Traffic Forecasting

The background traffic for 2023 was grown by 2%/year from the existing recorded volumes. The intersection volumes at the study area intersections are shown in Exhibit 12.

Total Traffic

New site trips were added to the "no Project" volumes. The resulting peak hour total traffic for 2023 are shown in Exhibits 13. These peak hour volumes were used to analyze future intersection operations. The analysis is provided in the next section.

Exhibit 11 Site Traffic Assignment



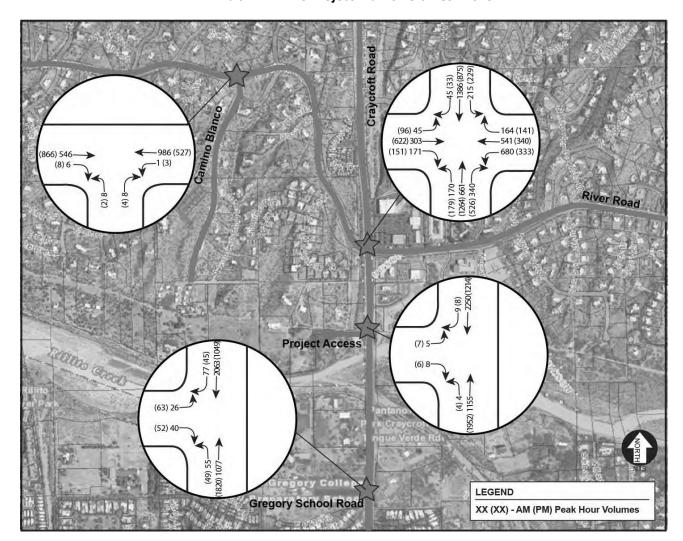


Exhibit 12 No Project Traffic Volumes - 2023

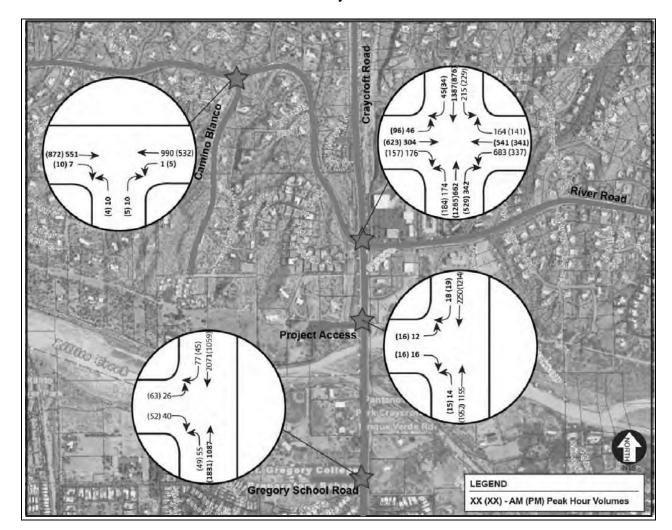


Exhibit 13 With Project Traffic Volumes - 2023

Traffic and Circulation Analysis

Roadway Performance

New daily site trips were added to the 2023 background volumes to analyze roadway directional performance with the additional trips. The analysis summarized in Exhibit 14 shows that both segments of River Road, and Craycroft Road south of River Road will continue to operate over their LOS D capacities even without the Project through 2023. Craycroft Road will operate below its LOS D daily capacity with the Project.

Exhibit 14 Roadway Volumes (2023)

Roadway Segment	Year	LOS D Capacity	Recorded ADT	2023 No Project ADT	Site Trips	2023 With Project	2023 No Project Over/Under LOS D Capacity	2023 With Project Over/Under LOS D Capacity
Craycroft Road, North of River Road	2019	35,820	30,151	32,636	56	32,692	Under	Under
Craycroft Road, North of Project Driveway	2020	35,820	37,603	39,905	196	40,100	Over	Over
Craycroft Road, South of Project Driveway	2020	35,820	37,545	39,843	196	40,039	Over	Over
River Road, East of Craycroft Road	2019	13,320	16,017	17,337	75	17,412	Over	Over
River Road, West of Craycroft Road	2019	13,320	13,920	15,067	131	15,198	Over	Over

Intersection Performance

The study area intersections were analyzed under "without Project" and "with Project" conditions for the year 2023. The am and pm peak hour analyses results are provided in Exhibits 15 and 16 for the off-site intersections.

2023 No Project

The following intersections and movements will operate at LOS E or F under the 2023 No Project Conditions:

River Road/Craycroft Road

- Eastbound Right, LOS E, AM
- Westbound Left LOS E, PM
- Northbound Left, LOS F, AM
- Northbound Through and Approach, LOS F, PM
- Southbound Through and Approach, LOS F, AM
- Southbound Left and Through, LOS F, PM
- Southbound Approach, LOS E, PM
- Intersection, LOS F, AM
- Intersection, LOS E, PM

Craycroft/Main Access Road

- Eastbound Left, LOS F, AM
- Eastbound Left, LOS E, PM

River Road/Camino Blanco

Northbound Left, LOS E, AM

2023 With Project

The following intersections and movements will operate at LOS E or F under the 2023 With Project Conditions

River Road/Craycroft Road

- Eastbound Right, LOS E, AM
- Westbound Left LOS E, PM
- Northbound Left, LOS F, AM
- Northbound Through and Approach, LOS F, PM
- Southbound Through and Approach, LOS F, AM
- Southbound Left and Through, LOS F, PM
- Southbound Approach, LOS E, PM
- Intersection, LOS F, AM
- Intersection, LOS E, PM

Craycroft/Main Access Road

- Eastbound Left, LOS F, AM
- Eastbound Left, LOS E, PM

River Road/Camino Blanco

Northbound Left, LOS E, AM and PM

River Road/Gregory School will continue to operate at LOS D or better with the Project during the peak hours.

For Parcel A, with only access to Craycroft Road, the eastbound left on the Main Access Road will experience longer delays during both peak hours than under the no Project condition. For Parcel B, with only access to River Road, the northbound approach on Camino Blanco at its intersection with River Road will experience longer delays during both peak hours than under the no Project condition, but the relative impact would not be significant.

Exhibit 15 2023 Intersection Performance – No Project

River/Craycroft

	No Project (2022)							
	AM		PM					
	Delay		Delay					
	(sec/veh)	LOS	(sec/veh)	LOS				
Eastbound								
Left	38.8	D	32.7	С				
Through	44.5	D	51.3	D				
Right	58	Е	33.2	С				
Approach	48.4	D	46.1	D				
Westbound								
Left	53.2	D	55.7	Е				
Through	40.6	D	39.2	D				
Right	37.9	D	40.4	D				
Approach	46.5	D	46.1	D				
Northbound								
Left	101.9	F	36.7	D				
Through	41	D	121.9	F				
Right	19.4	В	31.4	C F				
Approach	43.6	D	90	F				
Southbound								
Left	49.4	D	96	F				
Through	194.3	F	61.3	F				
Right	23.1	С	23.1	С				
Approach	170.7	F	67.2	E				
Intersection	93.1	F	69.4	Ε				

Craycroft/Gregory School

	No	Proje	ect (2022)	
	AM		PM	
	Delay		Delay	
	(sec/veh)	LOS	(sec/veh)	LOS
Eastbound				
Left	46.6	D	21.8	С
Right	53.6	D	21.7	С
Approach	50.9	D	21.8	С
Northbound				
Left	24.3	С	11.1	В
Through	2.2	Α	26.1	С
Approach	3.3	Α	25.8	С
Southbound				
Through	16.3	В	19.5	В
Right	16.9	В	19.4	В
Approach	16.6	В	19.5	В
Intersection	13.1	В	23.4	С

Craycroft/Project Driveway

	No Project (2022)							
	AM		PM					
	Delay		Delay					
	(sec/veh)	LOS	(sec/veh)	LOS				
Eastbound								
Left	148.8	F	35.7	Е				
Right	32.3	D	13.8	В				
Northbound								
Left	28.1	D	11.7	В				

River Road/Camino Blanco

	No Project (2022)						
	AM		PM				
	Delay		Delay				
	(sec/veh)	LOS	(sec/veh)	LOS			
Westbound							
Left	8.7	Α	10.1	В			
Northbound							
Left	41.4	Е	34.8	D			
Right	12.4	В	17.0	С			

Exhibit 16 2023 Intersection Performance – With Project

River/Craycroft

	Wi	h Pro	ject (2023))
	AM		PM	
	Delay		Delay	
	(sec/veh)	LOS	(sec/veh)	LOS
Eastbound				
Left	38.7	D	32.7	С
Through	44.4	D	51.5	D
Right	59.4	Е	33.6	С
Approach	48.9	D	46.2	D
Westbound				
Left	53.9	D	57.4	Е
Through	40.8	D	39.1	D
Right	38.1	D	40.3	D
Approach	46.9	D	46.9	D
Northbound				
Left	109.9	F	37.2	D
Through	41.5	D	122.3	F
Right	19.6	В	31.7	С
Approach	45.2	D	90.1	F
Southbound				
Left	50	D	96	F
Through	197.8	F	61.6	F
Right	23.3	С	23.1	С
Approach	173.7	F	67.3	E
Intersection	94.6	F	69.6	E

Craycroft/Gregory School

	With Project (2023)						
	AM		PM				
	Delay		Delay				
	(sec/veh)	LOS	(sec/veh)	LOS			
Eastbound							
Left	46.6	D	21.8	С			
Right	53.6	D	21.7	С			
Approach	50.9	D	21.8	С			
Northbound							
Left	24.7	С	11.2	В			
Through	2.2	Α	27.2	С			
Approach	3.3	Α	26.8	С			
Southbound							
Through	16.6	В	19.7	В			
Right	17.2	В	19.6	В			
Approach	16.9	В	19.7	В			
Intersection	13.2	В	24.1	С			

Craycroft/Project Driveway___

	With Project (2023)						
	AM		PM				
	Delay		Delay				
	(sec/veh)	LOS	(sec/veh)	LOS			
Eastbound							
Left	320.4	F	43.7	Е			
Right	27.5	Е	14.3	В			
Northbound							
Left	30.8	D	11.9	В			

River Road/Camino Blanco

	With Project (2023)				
	AM		PM		
	Delay	Delay			
	(sec/veh)	LOS	(sec/veh)	LOS	
Westbound					
Left	8.7	Α	10.2	В	
Northbound					
Left	42.6	Е	36.6	Е	
Right	12.4	В	17.2	С	

Queue and Storage Lengths

Projected maximum (95th percentile) queue lengths for the year 2023 are provided in in Exhibit 17 for the Craycroft/Project Access intersection.

These lengths are calculated by the Synchro program used for the intersection capacity analysis. The results indicate that the projected 95th percentile queue lengths will be shorter than the minimum storage lengths required for warranted turn lanes. Left turns are made on northbound Craycroft from the existing two-way left turn lane. It is recommended that Craycroft Road be restriped on the approach to the Craycroft Road/Main Access Road intersection access to delineate a 150-foot northbound left turn lane.

Exhibit 17 Queue and Storage Lengths – 2023 With Project

Craycroft/Main Access Road		2023 With Project		
	95th Percentile Que Existing (ft)			
	Storage (ft)	AM Peak Hour	PM Peak Hour	
Eastbound				
Left	N/A	78	23	
Right	N/A	23	5	
Northbound				
Left	N/A	15	3	

COT Minimum Storage Length = 110' for all roads with speed limit of 40 MPH or lower and 150' for roads with speed limits over 40 MPH

Potential Mitigation

The primary access to Parcel A will be from Craycroft Road. The Main Access Road must be improved to provide two-way traffic. Because access to the park and to residential uses to the north intersect the Main Access Road, it is recommended that these roads be stop-sign controlled at their intersections with the Main Access Road. The Main Access Road should be constructed to accommodate emergency vehicles.

At the intersection of Craycroft Road/Main Access Road, there will be delays for drivers entering Craycroft Road during the peak hours, and possibly at other hours. To reduce delays, the entrance should be reconstructed at the Craycroft Road/Main Access Road intersection with a restriction of right-out only for motor vehicle traffic. Outbound drivers wishing to head north on Craycroft Road but restricted to right-out only would be served by a new U-turn lane downstream at the Craycroft/Gregory School intersection.

The northbound Craycroft approach to the Craycroft Road/Main Access Road intersection should be also restriped to provide a 150-foot left turn lane, with pavement markings designed to City of Tucson standards.

Turn Lane Requirements

Turn lane warrants are contained in both the *Transportation Access Management Guidelines* for the City of Tucson⁵ and the Pima County Subdivision and Development Street Standards. The graph indicating the warrant thresholds based on volume and speed limit for four-lane roadways is provided in Exhibit 18.

Under the 2023 with Project scenario a southbound right turn lane is warranted at the Craycroft/Main Access Road intersection. Based on the queuing analysis there will be a need for less than 150 feet of storage, the City of Tucson's minimum turn lane length for roads with a 45-mph speed limit. The provision of this turn lane should consider whether sight distance would be reduced for drivers turning out of the Main Access Road with the construction of this turn lane.

The northbound Craycroft approach to the Craycroft Road/Main Access Road intersection should be restriped to delineate a left turn lane. The left turn lane should be striped for 150 feet of storage and be designed to City of Tucson pavement marking standards.

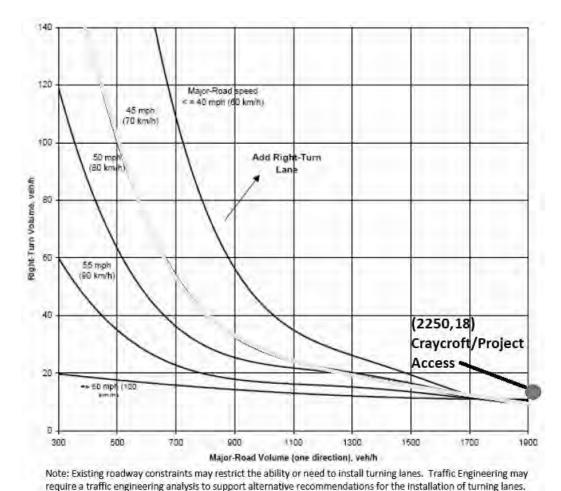


Exhibit 18 Right Turn Lane Warrant Criteria (4-lane Roadways)

Note: Highest SB Right Peak Hour Volume at Craycroft/Project Access (AM Peak Hour, 2023 With Project)

⁵ Transportation Access Management Guidelines for the City of Tucson, Arizona, page 26.



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Traffic Control Needs

The existing and future existing traffic at the Main Access Road on Craycroft Road face long delays representing LOS E or F conditions during commute hours. Drivers exiting the site wishing to go northbound on Craycroft Road must wait to turn left until there are gaps in traffic on a road that will carry close to 40,000 vehicles per day by the year 2023. This may be more difficult for the older drivers who may want to patronize businesses north of the Project site.

In order to facilitate left turns out at this location, the following options have been considered:

1. Signalization – Providing a signal at this location would need to be based on the results of a traffic signal warrant analysis, as well as a consideration of the spacing of other signalized intersections on Craycroft Road. The Manual of Uniform Traffic Control Devices provides guidelines and standards for signal warrant studies. Signals are typically warranted based on existing or projected hourly volume and volume thresholds. The traffic volumes coming out of the Project site on the Main Access Road would not meet any of the MUTCD signal warrant thresholds for a signal at this location. Other warrants (pedestrian, crash history) would also not be met.

The closest signalized intersections are at Craycroft/River and at Craycroft/Gregory School. The Craycroft/River intersection is approximately 970 feet north of the Project access and the Craycroft/Gregory School intersection is about 1,850 feet south of the Project access on Craycroft. The City of Tucson requires ½ mile spacing between signals on City arterials and collectors. The City does allow for non-standard spacing of signalized intersections, but one of the criteria is that the volume-based warrants be 1.5 times the standard thresholds. The projected volumes from the Main Access Road would not meet this criterion to allow for a signal at this location.

2. The City allows for the consideration of a Florida-T intersection where typical signalized intersection spacing cannot be met. We considered this for the Craycroft/Main Access Road intersection. However, the location of an existing full-access driveway on the east side of Craycroft Road about 230 feet north of the Main Access Road would create a conflict as outbound left turning vehicles at this east driveway may collide with the outbound northbound vehicles that may occupy the same space on Craycroft Road because of the close spacing of the two driveways.

With a no-left-turn restriction, drivers who turn right and then return to the north would need to have a convenient and safe downstream U-turn opportunity. At other similar locations, drivers have opportunities to turn right out of similar access locations with downstream U-turn locations nearby but there are no nearby downstream U-turn locations south on Craycroft Road.

On Craycroft Road, the closest U-turn lane would be at the Craycroft Road/Grant Road intersection, almost one and one-half miles south of the Main Access Road. There are other side streets that are available for drivers to turn into, and then return to the north, but these maneuvers are not recommended due to the number of conflicting movements required.

In order to provide this opportunity, a raised median can be constructed on the north leg of the Craycroft Road/Gregory School to better separate southbound to northbound U-turn movements from northbound through traffic at this signalized intersection. A concept of this is shown in Exhibit 19.

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Signal operation may need to be updated to accommodate the new U-turn movement at the intersection. It would also require the restriping of the southbound lanes to delineate the U-turn lane.

All traffic control devices, including signs and markings, should be installed in accordance with the MUTCD.

Exhibit 19 Left Turn/Raised Median Concept at Craycroft/Gregory School



Pedestrian, Bicycle, and Transit Considerations

Sidewalks and bike lanes will remain along the Parcel A Project frontage on Craycroft Road. Access to the County linear park will be provided for residents and staff from the site. There are no plans to add transit routes on Craycroft Road at this location.

Speed Considerations

Speeding is not known to be problematic in the Project vicinity.

Sight Distances

Sight visibility triangles will be included in the Project development plan, as required by the City of Tucson's development code.

Other

Access from River Road via Camino Blanco

Parcel B project access will only be to the north via River House Road and Camino Blanco toward River Road. This route would also be a secondary access for Parcel A in the event of a blockage on the Main Access Road. There would be a gated fence disallowing regular access from Parcel A.

Access Requirements in the Pima County Subdivision and Development Street Standards

The Pima County Subdivision and Development Street Standards includes requirements for the number of access locations dependent on number of units. For this project, three access points are required and two are provided. Pima County allows for Modifications of Standards when strict compliance with the Standards may be infeasible. Pima County staff is aware of the topographic and ownership constraints that will not permit a third access point. As such, Pima County staff acknowledges that a Modification of Standards will be requested at the time of Development Package.

Conclusions and Recommendations

- The proposed Parcel A project will generate about 391 trips during the average weekday, with about 14 during the morning commuter peak hour and 32 during the afternoon/evening commuter peak hour⁶. During the times when Parcel A will generate the highest morning and afternoon/evening hourly trips, the project will generate about 34 AM trips and 41 PM trips.
- The proposed Parcel B project will generate about 66 new trips during the average weekday, with about 5 during the morning peak hour and 7 during the afternoon/evening peak hour.

Current conditions:

- The intersection of River/Craycroft currently operates at LOS E or F during the morning and afternoon peak hours.
- The eastbound left turn movement at the Main Access Road intersection on Craycroft Road operates at LOS F during the morning peak hours and delays will increase without and with the Project through 2023. Delays are common for driveways or minor streets that enter major streets during peak commute hours.
- The intersection of Gregory School/Craycroft currently operates at LOS C during the peak hours and will continue to operate at LOS C through 2023 with the Project.
- o The northbound left turn movement at the River Road/Camino Blanco intersection operates at LOS E during the morning peak hour and will continue to operate at LOS E/F through 2023.
- o The Main Access Road on Craycroft Road exists and meets City of Tucson standards for driveway spacing and corner clearance.
- Adding the Project site trips to the future years' background traffic volumes will increase
 delays even more at the intersections, although the proportional increases are minor.

Parcel A recommended mitigation:

- Specific Project related intersection mitigation recommendations includes:
 - Re-striping the two-way left turn lane on the northbound Craycroft Road approach to the Craycroft Road/Main Access Road intersection to delineate a 150-foot left turn lane.
 - O Provide a warranted southbound right turn lane on Craycroft Road for turns into the Main Access Road. This improvement should only be considered if the sight distance for drivers entering Craycroft Road from the Main Access Road is not reduced to an unacceptable distance because of the improvement.
 - Reconstructing the Main Access Road to accommodate the lane configuration near Craycroft as shown in the site plan. Outbound traffic is restricted to right-out turning movements.
 - Providing stop control inside the reconstructed Main Access Road at the internal intersecting roads between the Rillito River Park trail head and the new Project/residences to the north.

⁶ Commuter peak hours are typically the highest volume one hour period in the 7-9 AM morning time frame and the highest volume one hour period in the 4-6 PM time frame.



© 2022

- Drivers turning left out of the Main Access Road today experience delays representative of LOS E or LOS F conditions. Elderly (or any) drivers wishing to head north of Craycroft Road from the Main Access Road would likely opt to turn right from the Main Access Road onto Craycroft and seek a downstream opportunity to turn around and head north. It is recommended that outbound movements be restricted to right turns out only to eliminate the potential for eastbound to northbound left turn crashes. The following improvement at the downstream intersection of Craycroft Road/Gregory School is recommended.
- To assist exiting drivers from the Main Access Road onto Craycroft Road, a new raised median delineated U-turn lane is recommended at the Craycroft Road/Gregory School access for the north leg of the intersection. The purpose of this is to provide a southbound U-turn lane at the intersection for use by drivers whose destinations are north of the Project access driveway and who would be restricted to the recommended right-out only movement at the Main Access Road intersection with Craycroft Road. The north leg of the Craycroft Road/Gregory School intersection would be reconstructed and restriped to provide a 150-foot left turn lane with a raised median separating northbound and southbound traffic. If this recommendation is approved, a left turn phase warrant analysis should be conducted for the southbound left turn lane.

Parcel B recommended mitigation:

 Parcel B trips will be only via Camino Blanco to River Road. Access from Parcel A to Camino Blanco will be only for emergencies through a gated access on the west side of the project. No additional mitigation is recommended on River House Road, Camino Blanco or River Road.

APPENDIX

- Site Plan
- Traffic Data
- Synchro Analysis





Intersection Turning Movement Prepared by:





N-S STREET: Craycroft Rd. DATE: 02/25/20 LOCATION: Tucson

E-W STREET: River Rd. DAY: TUESDAY PROJECT# 20-1100-001

	NC	ORTHBO	UND	SC	OUTHBOU	JND	E	ASTBOU	IND	W	ESTBOL	JND	
LANES:	NL 1	NT 2	NR 1	SL 2	ST 2	SR 1	EL 1	ET 2	ER 1	WL 2	WT 2	WR 1	TOTAL
6:00 AM 6:15 AM 6:30 AM 6:45 AM 7:00 AM 7:15 AM 7:30 AM 7:45 AM 8:00 AM 8:15 AM 8:30 AM 8:45 AM 9:00 AM 9:15 AM 9:30 AM 9:45 AM 10:00 AM 10:15 AM 10:30 AM 10:45 AM 11:30 AM	41 43 43 36 66 60 65 58	169 166 145 155 98 99 122 103	87 85 86 69 66 60 65 58	58 54 50 45 40 44 43 41	303 328 412 289 288 255 243 239	6 9 13 15 14 10 11 8	8 11 10 14 12 8 5 9	54 80 88 69 76 74 58 65	28 41 45 50 54 52 43 45	166 196 141 151 154 161 166 122	128 133 131 128 124 111 119 112	41 54 30 33 29 42 41 54	1089 1200 1194 1054 1021 976 981 914

TOTAL	NL	NT	NR	SL	ST	SR	EL	ET	ER	WL	WT	WR	TOTAL
Volumes	412	1057	576	375	2357	86	77	564	358	1257	986	324	8429
Approach %	20.15	51.69	28.17	13.31	83.64	3.05	7.71	56.46	35.84	48.97	38.41	12.62	
App/Depart	2045	1	1458	2818	1	3972	999	/	1515	2567	1	1484	

AM Peak Hr Begins at: 700 AM

PEAK Volumes Approach % 2023 NP Site Trips 2023 WP		163 14.49 170 4 174	635 56.44 661 1 662	327 29.07 340 2 342	207 13.08 215 215	1332 84.20 1386 1 1387	43 2.72 45 0 45	43 8.63 45 1 46	291 58.43 303 1 304	164 32.93 171 5 176	654 49.10 680 3 683	520 39.04 541 0 541	158 11.86 164 164	4537	
PEAK HR. FACTOR:	I		0.947	ı		0.833	1		0.871	I		0.869	ı	0.945	I

CONTROL: Signal

COMMENT 1:

GPS: 32.273464, -110.875095

Intersection Turning Movement



N-S STREET: Craycroft Rd. DATE: 02/25/20 LOCATION: Tucson

0

E-W STREET: River Rd. DAY: TUESDAY PROJECT# 20-1100-001

	NC	RTHBO	UND	SO	UTHBOU	JND	E	ASTBOU	ND	W	ESTBOU	ND	
LANES:	NL 1	NT 2	NR 1	SL 2	ST 2	SR 1	EL 1	ET 2	ER 1	WL 2	WT 2	WR 1	TOTAL
1:00 PM													
1:15 PM													
1:30 PM													
1:45 PM													
2:00 PM													
2:15 PM													
2:30 PM													
2:45 PM													
3:00 PM													
3:15 PM													
3:30 PM													
3:45 PM	4.0	200	100	F/	٥٦٦	4	10	1.40	20	00	0.5	40	1101
4:00 PM	43	299	128	56	255	4	18	142	28	80	85	43	1181
4:15 PM	44	296	122	52	196	7	22	141	41	69	88	41	1119
4:30 PM	40	298	131	58	199	10	20	154	43	86	80	24	1143
4:45 PM	45	322	125	54	191	11	32	161	33	85	74	28	1161
5:00 PM	50 54	325	124	50	214	7	25	166	30 32	45	76 40	24	1136 1072
5:15 PM 5:30 PM	54 41	274 269	141 143	45 43	198 185	5 8	24 21	169 131	32 28	41 50	69 66	20 22	1072
5:45 PM	41	246	130	43 41	147	o 5	19	122	24	55	60	19	910
6:00 PM	42	240	130	41	147	5	17	122	24	55	00	17	910
6:15 PM													
6:30 PM													
6:45 PM													
OTAL	NL	NT	NR	SL	ST	SR	EL	ET	ER	WL	WT	WR	TOTAL
olumes	359	2329	1044	399	1585	57	181	1186	259	511	598	221	8729
proach %	9.62	62.41	27.97	19.55	77.66	2.79	11.13	72.94	15.93	38.42	44.96	16.62	
n/Donort	2722	1	2721	2041	,	2255	1424	/	2420	1220	,	1014	

 Approach %
 9.62
 62.41
 27.97
 19.55
 77.66
 2.79
 11.13
 72.94
 15.93
 38.42
 44.96
 16.62

 App/Depart
 3732
 /
 2731
 2041
 /
 2355
 1626
 /
 2629
 1330
 /
 1014

PM Peak Hr Begins at: 400 PM

PEAK															
Volumes	1	172	1215	506	220	841	32	92	598	145	320	327	136	4604	I
Approach %		9.09	64.18	26.73	20.13	76.94	2.93	11.02	71.62	17.37	40.87	41.76	17.37		
2023 NP		179	1264	526	229	875	33	96	622	151	333	340	141		
Site Trips		5	1	3		1	1	0	1	6	4	1			
2023 WP		184	1265	529	229	876	34	96	623	157	337	341	141		
PEAK HR.				_						_			_		_
FACTOR:			0.962			0.867			0.924			0.941		0.975	

CONTROL: Signal COMMENT 1: 0

GPS: 32.273464, -110.875095

Intersection Turning Movement re re





N-S STREET: Craycroft Rd. DATE: 02/25/20 LOCATION: Tucson

E-W STREET: Pro ect Access DAY: TUESDAY PROJECT# 20-1100-002

	NO	ORTHBO	UND	SC	OUTHBO	JND	E	ASTBOL	JND	W	/ESTBOL	JND	
LANES:	NL 0	NT 2	NR 0	SL 0	ST 2	SR 0	EL 0	ET 1	ER 0	WL 0	WT 0	WR 0	TOTAL
6:00 AM													
6:15 AM													
6:30 AM													
6:45 AM													
7:00 AM	2	304	0	0	483	3	2	0	1	0	0	0	795
7:15 AM	0	300	0	0	581	1	0	0	1	0	0	0	883
7:30 AM	0	262	0	0	623	4	2	0	0	0	0	0	891
7:45 AM	2	244	0	0	476	1	1	0	6	0	0	0	730
8:00 AM	1	216	0	0	479	2	1	0	2	0	0	0	701
8:15 AM	4	223	0	0	469	0	0	0	2	0	0	0	698
8:30 AM	2	278	0	0	430	6	4	0	3	0	0	0	723
8:45 AM	2	211	0	0	410	4	1	0	2	0	0	0	630
9:00 AM													
9:15 AM													
9:30 AM													
9:45 AM													
10:00 AM													
10:15 AM													
10:30 AM													
10:45 AM													
11:00 AM													
11:15 AM													
11:30 AM													
11:45 AM													

TOTAL	NL	NT	NR	SL	ST	SR	EL	ET	ER	WL	WT	WR	TOTAL
Volumes	13	2038	0	0	3951	21	11	0	17	0	0	0	6051
Approach %	0.63	99.37	0.00	0.00	99.47	0.53	39.29	0.00	60.71	####	####	####	
App/Depart	2051	/	2049	3972	/	3968	28	/	0	0	/	34	

AM Peak Hr Begins at: 700 AM

PEAK														
Volumes	4	1110	0	0	2163	9	5	0	8	0	0	0	3299	
2023 NP	4	1155	0	0	2250	9	5	0	8				ı	
Site Trips	10					9	7		8				ı	
2023 WP	14	1155	0	0	2250	18	12	0	16				İ	
													Í	
Approach %	0.36	99.64	0.00	0.00	99.59	0.41	38.46	0.00	61.54	####	####	####	İ	

PEAK HR.

FACTOR: 0.910 0.866 0.464 0.000 0.926

CONTROL: 1-Way Stop EB

COMMENT 1:

GPS: 32.270825, -110.875100

Intersection Turning Movement

FIELD DATA SERVICES OF ARIZONA, INC. Veracitytrafficgroup

N-S STREET: Craycroft Rd. DATE: 02/25/20 LOCATION: Tucson

E-W STREET: Pro ect Access DAY: TUESDAY PROJECT# 20-1100-002

	NO	ORTHBO	UND	SC	OUTHBOU	JND	E	ASTBOL	IND	W	'ESTBO	JND	
LANES:	NL 0	NT 2	NR 0	SL 0	ST 2	SR 0	EL 0	ET 1	ER 0	WL 0	WT O	WR 0	TOTAL
1:00 PM													
1:15 PM													
1:30 PM													
1:45 PM													
2:00 PM													
2:15 PM													
2:30 PM													
2:45 PM													
3:00 PM													
3:15 PM													
3:30 PM													
3:45 PM													
4:00 PM	4	445	0	0	334	2	2	0	4	0	0	0	791
4:15 PM	0	444	0	0	286	1	1	0	0	0	0	0	732
4:30 PM	0	452	0	0	291	3	1	0	1	0	0	0	748
4:45 PM	3	472	0	0	303	3	2	0	2	0	0	0	785
5:00 PM	1	508	0	0	287	1	3	0	3	0	0	0	803
5:15 PM	0	452	0	0	268	0	1	0	3	0	0	0	724
5:30 PM	2	444	0	0	250	1	0	0	2	0	0	0	699
5:45 PM	3	392	0	0	236	0	1	0	2	0	0	0	634
6:00 PM													
6:15 PM													
6:30 PM													
6:45 PM													

TOTAL	NL	NT	NR	SL	ST	SR	EL	ET	ER	WL	WT	WR	TOTAL
Volumes	13	3609	0	0	2255	11	11	0	17	0	0	0	5916
Approach %	0.36	99.64	0.00	0.00	99.51	0.49	39.29	0.00	60.71	####	####	####	
App/Depart	3622	/	3620	2266	/	2272	28	/	0	0	/	24	

PM Peak Hr Begins at: 415 PM

PEAK														
Volumes	4	1876	0	0	1167	8	7	0	6	0	0	0	3068	l
2023 NP	4	1952	0	0	1214	8	7	0	6					l
Site Trips	11					11	9		10					l
2023 WP	15	1952	0	0	1214	19	16	0	16					l
														l
Approach %	0.21	99.79	0.00	0.00	99.32	0.68	53.85	0.00	46.15	####	####	####		l
DE ALC LID														
PEAK HR.														

0.542

0.960

0.955

0.000

FACTOR: 0.923

CONTROL: 1-Way Stop EB

COMMENT 1: 0

GPS: 32.270825, -110.875100

Intersection Turning Movement re re





N-S STREET: Camino Blanco DATE: 08/30/18 LOCATION: Tucson

E-W STREET: East River Rd. DAY: THURSDAY PROJECT# 18-1387-001

LANES: 0 1 0 0 0 0 0 1 0 0 1 0 0 1 0 6:00 AM 6:15 AM 6:30 AM 6:45 AM 7:00 AM 1 0 0 0 0 0 0 0 0 102 3 0 202 0 30 7:15 AM 0 0 3 0 0 0 0 115 0 0 243 0 36 7:30 AM 1 0 2 0 0 0 0 0 115 0 0 243 0 36 7:30 AM 1 0 2 0 0 0 0 0 139 1 0 210 0 35 7:45 AM 3 0 1 0 0 0 0 0 120 1 1 233 0 35 8:00 AM 3 0 1 0 0 0 0 0 130 4 0 225 0 36 8:15 AM 3 0 4 0 0 0 0 0 142 1 0 170 0 32 8:30 AM 0 0 6 0 0 0 0 0 105 1 0 128 0 23														
LANES: 0 1 0 0 0 0 0 1 0 0 1 0 0 1 0 6:00 AM 6:15 AM 6:30 AM 6:45 AM 7:00 AM 1 0 0 0 0 0 0 0 0 102 3 0 202 0 30 7:15 AM 0 0 3 0 0 0 0 115 0 0 243 0 36 7:30 AM 1 0 2 0 0 0 0 0 115 0 0 243 0 36 7:30 AM 1 0 2 0 0 0 0 0 139 1 0 210 0 35 7:45 AM 3 0 1 0 0 0 0 0 120 1 1 233 0 35 8:00 AM 3 0 1 0 0 0 0 0 130 4 0 225 0 36 8:15 AM 3 0 4 0 0 0 0 0 142 1 0 170 0 32 8:30 AM 0 0 6 0 0 0 0 0 105 1 0 128 0 23		NO	ORTHBO	UND	SO	UTHBO	UND	E	ASTBOU	ND	W	ESTBOU	JND	
6:15 AM 6:30 AM 6:45 AM 7:00 AM 1 0 0 0 0 0 0 0 102 3 0 202 0 30 7:15 AM 0 0 0 115 0 0 243 0 36 7:30 AM 1 0 2 0 0 0 0 139 1 0 210 0 35 7:45 AM 3 0 1 0 0 0 0 0 120 1 1 233 0 35 8:00 AM 3 0 1 0 0 0 0 0 130 4 0 225 0 36 8:15 AM 3 0 4 0 0 0 0 0 142 1 0 170 0 32 8:30 AM 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	LANES:													TOTAL
9:00 AM 9:15 AM 9:30 AM 9:45 AM 10:00 AM 10:15 AM 10:30 AM 10:45 AM	6:15 AM 6:30 AM 6:45 AM 7:00 AM 7:15 AM 7:30 AM 7:45 AM 8:00 AM 8:15 AM 9:30 AM 9:15 AM 9:30 AM 9:45 AM 10:00 AM	0 1 3 3 3 0	0 0 0 0 0	3 2 1 1 4 6	0 0 0 0 0	0 0 0 0 0	0 0 0 0 0	0 0 0 0 0	115 139 120 130 142 97	0 1 1 4 1 2	0 0 1 0 0	243 210 233 225 170 172	0 0 0 0 0	308 361 353 359 363 320 278 238

TOTAL	NL	NT	NR	SL	ST	SR	EL	ET	ER	WL	WT	WR	TOTAL
Volumes	14	0	18	0	0	0	0	950	13	2	1583	0	2580
Approach %	43.75	0.00	56.25	####	####	####	0.00	98.65	1.35	0.13	99.87	0.00	
App/Depart	32	/	0	0	/	15	963	/	968	1585	/	1597	

AM Peak Hr Begins at: 715 AM

PEAK Volumes Approach %	7 50.00	0 0.00	7 50.00	0 ####	0 ####	0 ####	0 0.00	504 98.82	6 1.18	1 0.11	911 99.89	0 0.00	1436	
2020 E isting 2023 NP Site Trips 2023 WP	7 8 2.00 10	0 0	7 8 2.00 10	0 0	0 0	0 0	0 0	524 546 5.00 551	6 6 1.00 7	1 1 0.00 1	948 986 4.00 990	0 0		
PEAK HR. FACTOR:	l	0.875	ļ		0.000	1		0.911	I		0.938	1	0.989	I

CONTROL: 1-Way Stop NB

COMMENT 1:

GPS: 32.279366, -110.880450

Intersection Turning Movement



N-S STREET: Camino Blanco DATE: 08/30/18

LOCATION: Tucson

E-W STREET: East River Rd.

DAY: THURSDAY

PROJECT# 18-1387-001

	NO	RTHBO	UND	SC	OUTHBO	UND	E <i>F</i>	ASTBOU	ND	W	ESTBOU	ND	
LANES:	NL O	NT 1	NR 0	SL 0	ST 0	SR 0	EL 0	ET 1	ER 0	WL O	WT 1	WR 0	TOTAL
1:00 PM													
1:15 PM													
1:30 PM													
1:45 PM													
2:00 PM													
2:15 PM													
2:30 PM													
2:45 PM													
3:00 PM													
3:15 PM													
3:30 PM													
3:45 PM													
4:00 PM	1	0	2	0	0	0	0	195	2	0	113	0	313
4:15 PM	0	0	0	0	0	0	0	187	2	0	101	0	290
4:30 PM	0	0	0	0	0	0	0	153	2	3	90	0	248
4:45 PM	0	0	1	0	0	0	0	176	3	0	133	0	313
5:00 PM	0	0	3	0	0	0	0	223	1	1	126	0	354
5:15 PM	1	0	0	0	0	0	0	193	2	2	110	0	308
5:30 PM	1	0	0	0	0	0	0	208	1	0	118	0	328
5:45 PM	2	0	2	0	0	0	0	146	3	5	90	0	248
6:00 PM													
6:15 PM													
6:30 PM													
6:45 PM													
TAL	NL	NT	NR	SL	ST	SR	EL	ET	ER	WL	WT	WR	TOTA
lumes	5	0	8	0	0	0	0	1481	16	11	881	0	2402
proach %	38.46	0.00	61.54	####	####	####	0.00	98.93	1.07	1.23	98.77	0.00	
n/Denart	13	/	0	0	/	27	1497	/	1489	892	/	886	

PM Peak Hr Begins at: 445 PM

PEAK Volumes Approach %	2 33.33	0 0.00	4 66.67 #	0 ####	0 ####	0 ####	0 0.00	800 99.13	7 0.87	3 0.61	487 99.39	0 0.00	03	
2020 E isting	2	0	4	0	0	0	0	832	7	3	507	0		
2023 NP	2	0	4	0	0	0	0	866	8	3	527	0		
Site Trips	2.00		1.00					6.00	2.00	2.00	5.00			
2023 WP	4	0	5	0	0	0	0	872	10	5	532	0		

CONTROL: 1-Way Stop NB

COMMENT 1: 0

GPS: 32.279366, -110.880450

Date: Thursday, September 27, 2018 Project: Craycroft/St Gregorys

Count period: 0:15

											8:00 AM	8:15 AM	8:30 AM	8:45 AM	9:00 AM	9:00 AM																5:00 PM	5:15 PM	5:30 PM	5:45 PM	6:00 PM	$6:00 \mathrm{PM}$					
	END	Time	7:15 AM	7:30 AM	7:45 AM	8:00 AM	8:15 AM	8:30 AM	8:45 AM	9:00 AM	7:00 AM	7:15 AM	7:30 AM	7:45 AM	8:00 AM	7:00 AM							END	4.15 PM	4:30 PM	4:45 PM	5:00 PM	5:15 PM	5:30 PM	5:45 PM	6:00 PM	4:00 PM	4:15 PM	4:30 PM	4:45 PM	5:00 PM	4:00 PM					
		Total	605	841	801	736	902	703	674	969		-	2946	2819	2679	5995							Total	10tal 645	189	652	703	693	757	069	531	2681	2729	2805	2843	2671	5352					
		WB	0	0	0	0	0	0	0	0	0	0	0	0	0	0							WB	٠ ا	0	0	0	0	0	0	0	0	0	0	0	0	0					
	TOTALS	EB	7	7	20	16	18	27	58	34	50	19	81	119	137	187	92.0						TOTALS	17	15	20	19	25	38	24	15	99	62	102	106	102	891	0.70				
	•	SB	356	523	539	472	443	436	393	373	0681	1977	1890	1744	1645	3535	0.92						as	263	267	238	244	259	254	254	180	1012	1008	962	1011	947	1959	86.0				
	-	NB	242	311	242	248	245	240	223	189	1043	1046		956		_	0.84						AN AN	370	399	394	440	409	+		336	1603		1708	1726	1622	3225	0.93				
L											<u> </u>						PHF				i					<u> </u>												PHF				
		Turn	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0		0			Right		0	0	0	0	0	0	0	0	0	0	0	0	0		0	0		
		THRU	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0		0		River	THBII	ONIII	0	0	0	0	0	0	0	0	0	0	0	0	0		0	0		
1	Left	Turn	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0		0		WB	Left	T O	0	0	0	0	0	0	0	0	0	0	0	0	0		0	0		
	j.	Turn	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0		0			Ų.	T O	0	0	0	0	0	0	0	0	0	0	0	0	0		0	0		
	Right	Turn	2	5	12	12	8	12	28	19	31	37	4	09	<i>L</i> 9	86	38	40		40			Right	1 mm (1 (1)	7	5	13	19	11	8	17	28	44	48	51	89		50	52		
1.25.7		THRU	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0		0		EB St Gregorys	ТИВП	ONITIO	0	0	0	0	0	0	0	0	0	0	0	0	0		0	0		
	Left	Turn	5	2	8	4	10	15	30	15	19	24	37	65	70	68	25	26		26		EB St (Left	101	12	13	14	12	19	13	7	46	51	28	28	51	100		09	63		
	j	Turn	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0		0			Ų.	T O	0	0	0	0	0	0	0	0	0	0	0	0	0		0	0		
		Turn	8	15	16	20	20	31	61	18	65	71	87	132	130	189	74	77		77			Right		23	18	6	10	15	8	6	09	09	52	42	42	102		4	45		
arana arana		THRU	348	208	523	452	423	405	332	355	1831	1906	1803	1612	1515	3346	1983	2063	∞	2071		SB Craycroft	THPII	253	244	220	235	249	239	246	171	952	948	943	696	905	1857		1008	1049	10	
	Left	Turn	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0		0		SBCr	Left	O	0	0	0	0	0	0	0	0	0	0	0	0	0		0	0		
	<u>-</u>	Turn	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0		0			J (O	0	0	0	0	0	0	0	0	0	0	0	0	0		0	0		
	Right	Turn	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0		0			Right	T O	0	0	0	0	0	0	0	0	0	0	0	0	0		0	0		
arara fara era		THRU	238	291	235	236	233	227	188	183	1000	968	931	884	831	1831	1035	1077	10	1087		NB Craycroft	THPII	357	386	380	427	402	448	404	333	1545	1595	1657	1891	1587	3132		1749	1820	Π	
	Left	Turn	4	20	7	12	12	13	35	9	43	51	4	72	99	109	53	55		55		NB Cr	Left	118	13	14	13	7	17	8	3	28	47	51	45	35	63		47	49		
	j.	Turn	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0		0	at		U-	0	0	0	0	0	0	0	0	0	0	0	0	0	0		0	0		
	END	Time	7:15 AM	7:30 AM	7:45 AM	8:00 AM	8:15 AM	8:30 AM	8:45 AM	9:00 AM	8:00 AM	8:15 AM	8:30 AM	8:45 AM	9:00 AM	9:00 AM	je-	0			S	4:00 PM	END	4.15 PM	4:30 PM	4:45 PM	5:00 PM	5:15 PM	5:30 PM	5:45 PM	$6:00 \mathrm{PM}$	$5:00 \mathrm{PM}$	5:15 PM	5:30 PM	5:45 PM	6:00 PM	$6:00 \mathrm{PM}$		b .c			
											7:00 AM	7:15 AM	7:30 AM	7:45 AM	8:00 AM	7:00 AM	2020 Existing	2023 NP	Site Trips	2023 WP	ర '											4:00 PM	4:15 PM	4:30 PM	4:45 PM	5:00 PM	4:00 PM		2020 Existing	2023 NP	Site Trips	

Prepared by: Field Data Services of Arizona/Veracity Traffic Group (520) 316-6745

Volumes for: Tuesday, Fe ruary 25, 2020 City: Tucson Pro ect #: 20-1100-003

Location: Craycroft Rd. north of Pro ect Access

AM Period	NB		SB		EB	WB		PM Period	NB		SB		EB		WB		
00:00	19		11					12:00	303		275						
00:15	6		5					12:15	380		267						
00:30	8		9					12:30	242		285						
00:45	15	48	8	33			81	12:45		1219	246	1073					2292
01:00	14		5					13:00	302		266						
01:15	5		7					13:15	293		284						
01:30	6	20	5	21			40	13:30	277	1101	277	1005					22//
01:45	3	28	4	21			49	13:45	309	1181	258	1085					2266
02:00	6		6					14:00	311		249						
02:15 02:30	4 8		3 4					14:15 14:30	307 359		259287						
02:30	6	24	4	17			41	14:45	385	1362	337	1132					2494
03:00	4		6					15:00	406		320						
03:15	5		8					15:15	460		312						
03:30	12		12					15:30	498		291						
03:45	6	27	16	42			69	15:45	471	1835	308	1231					3066
04:00	8		14					16:00	447		336						
04:15	15		23					16:15	445		287						
04:30	21		23					16:30	453		294						
04:45	24	68	30	90			158	16:45	474	1819	306	1223					3042
05:00	27		51					17:00	511		288						
05:15	38		58					17:15	453		268						
05:30	48		98					17:30	444		251						
05:45	55	168	95	302			470	17:45	393	1801	236	1043					2844
06:00	75		132					18:00	358		207						
06:15	106		182					18:15	346		196						
06:30	126	407	295	000			1207	18:30	368	1407	166	720					21//
06:45	180	487	300	909			1396	18:45	364	1436	161	730					2166
07:00	306		487					19:00	331		154						
07:15 07:30	300 264		582 627					19:15 19:30	287 245		147 141						
07:30		1115		2173			3288	19:45	205	1068	145	587					1655
08:00	217	1110	481	2170			0200	20:00	197	1000	152	007					1000
08:00	223		469					20:00	201		166						
08:30	282		436					20:30	162		161						
08:45	212	934		1800			2734	20:45	157	717	139	618					1335
09:00	212		338					21:00	143		133						
09:15	215		306					21:15	144		69						
09:30	210		332					21:30	137		80						
09:45	209	846	284	1260			2106	21:45	121	545	87	369					914
10:00	227		274					22:00	106		74						
10:15	238		283					22:15	88		76						
10:30	258		277					22:30	85		60						
10:45	249	972	167	1001			1973	22:45	87	366	65	275					641
11:00	232		276					23:00	68		54						
11:15	268		184					23:15	61		58						
11:30	274	1042	292	000			2050	23:30	66 50	2E /	52 55	210					172
11:45	200	1062	236	988			2050	23:45	59	254	55	219					473
Tot o		5779		8636						13603		9585					
GPS Coordi	nates	:	32	.271113,	-110.875090								i	Tot s			
										NB		SB		EB	\	WB	om ine
										19382		18221					
	_				M					F0 70:		14.00		M			
it		40.1%		59.9%						58.7%		41.3%					
e our		11:30		07:00						16:30		14:45					
o ume		1245		2173						1891		1260					
		0.82		0.87						0.93		0.93					

Prepared by: Field Data Services of Arizona/Veracity Traffic Group (520) 316-6745

Volumes for: Tuesday, Fe ruary 25, 2020 City: Tucson Pro ect #: 20-1100-004

Location: Craycroft Rd. south of Pro ect Access

AM Period	NB		SB		EB	WB		PM Period	NB		SB		EB	V	VB	
00:00	19		11					12:00	302		276					
00:15	7		5					12:15	376		265					
00:30	8		9					12:30	241		285					
00:45	15	49	8	33			82	12:45	293	1212	247	1073				2285
01:00	14		5					13:00	301		267					
01:15	5		7					13:15	292		284					
01:30	6		5					13:30	275		275					
01:45	3	28	4	21			49	13:45	308	1176	260	1086				2262
02:00	6		6					14:00	306		249					
02:15	4		3					14:15	308		258					
02:30	8	2.4	4	17			44	14:30	360	1250	288	1120				2407
02:45	6	24	4	17			41	14:45	384	1358	334	1129				2487
03:00	4		6					15:00	407		319					
03:15	5		8					15:15	457		312					
03:30 03:45	12 6	27	12 16	42			69	15:30 15:45	496 468	1828	294 308	1233				3061
		21		42			09			1020		1233				3001
04:00	8 1E		14					16:00	449		338					
04:15 04:30	15 22		23 23					16:15 16:30	444 452		286 292					
04:30	25	70	30	90			160	16:45	475	1820	305	1221				3041
05:00	29	70	50	70			100	17:00	509	1020	290	1221				0011
05:00	39		56					17:00	452		271					
05:30	48		96					17:13	446		252					
05:45	54	170	94	296			466	17:45	395	1802	238	1051				2853
06:00	71		133					18:00	356		206					
06:15	103		182					18:15	345		198					
06:30	126		295					18:30	366		167					
06:45	179	479	299	909			1388	18:45	363	1430	165	736				2166
07:00	306		484					19:00	328		155					
07:15	301		582					19:15	285		150					
07:30	262		623					19:30	244		143					
07:45		1116		2171			3287	19:45	201	1058	150	598				1656
08:00	217		481					20:00	196		154					
08:15	228		471					20:15	199		167					
08:30	280		433					20:30	161		165					
08:45	213	938	412	1797			2735	20:45	154	710	140	626				1336
09:00	212		335					21:00	141		135					
09:15	214		305					21:15	143		72					
09:30	210		330					21:30	133		82					
09:45	206	842	283	1253			2095	21:45	120	537	88	377				914
10:00	224		267					22:00	104		74					
10:15	233		282					22:15	87		77					
10:30	255		274					22:30	85		62					
10:45	249	961	167	990			1951	22:45	86	362	66	279				641
11:00	235		277					23:00	66		58					
11:15	265		186					23:15	60		59					
11:30	270	1050	287	007			2044	23:30	65	240	54	227				47/
11:45	288	1058	236	986			2044	23:45	58	249	56	227				476
Tot o		5762		8605						13542		9636				
GPS Coordi	nates	:	32.	.270449,	-110.875095								i	Tot s		
										NB		SB		EB	WB	om ine
										19304		18241				
					M			_						M		
it		40.1%		59.9%						58.4%		41.6%				
e our		11:30		07:00						16:30		14:45				
o ume		1236		2171						1888		1259				
		0.82		0.87						0.93		0.94				

Prepared by: Field Data Services of Arizona/Veracity Traffic Group (520) 316-6745

Volumes for: Tuesday, Fe ruary 25, 2020 City: Tucson Pro ect #: 20-1100-005

Location: Pro ect Access est of Craycroft Rd.

Location: Pro ect Academic AM Period NB	SB	EB		WB			PM Period	NB	SB		EB		WB		
00:00		0		0			12:00				5		3		
00:15		0		0			12:15				4		2		
00:30		0		1			12:30				4		3		
00:45		0	0	0	1	1	12:45				6	19	5	13	32
01:00		1		0			13:00				4		2		
01:00		0		0			13:00				2		1		
01:30		0		1			13:15				2		2		
01:45		0	1	2	3	4	13:45				5	13	2	7	20
					<u> </u>	4						13			
02:00		0		1			14:00				6		1		
02:15		1		0			14:15				2		4		
02:30		0	_	0	_		14:30				3		3	4.0	0.7
02:45		0	1	0	1	2	14:45				3	14	5	13	27
03:00		0		0			15:00				1		3		
03:15		0		0			15:15				5		2		
03:30		0		0			15:30				5		0		
03:45		0	0	0	0		15:45				5	16	2	7	23
04:00		0		0			16:00				6		6		
04:15		0		0			16:15				1		1		
04:30		0		1			16:30				2		3		
04:45		1	1	2	3	4	16:45				4	13	6	16	29
05:00		0		3			17:00	_	_	_	6		2	_	_
05:15		0		3			17:15				4		0		
05:30		0		2			17:30				2		3		
05:45		2	2	2	10	12	17:45				3	15	3	8	23
					10	12									
06:00		7		2			18:00				6		5		
06:15		6		3			18:15				3		5		
06:30		1	4.5	1	-	00	18:30				3	47	3	4.4	0.4
06:45		1	15	1	7	22	18:45				5	17	1	14	31
07:00		3		5			19:00				4		5		
07:15		1		1			19:15				5		1		
07:30		16		4			19:30				3		1		
07:45		7	27	3	13	40	19:45				9	21	3	10	31
08:00		3		3			20:00				3		3		
08:15		7		4			20:15				3		1		
08:30		7		8			20:30				5		1		
08:45		3	20	6	21	41	20:45				4	15	0	5	20
09:00		3		7			21:00				4		2		
09:15		4		4			21:15				4		0		
09:30		5		7			21:30				6		2		
09:45		5	17	4	22	39	21:45				2	16	2	6	22
			1 /			37						10			
10:00		3		7			22:00				2		1		
10:15		6		2			22:15				2		0		
10:30		5	4.0	5	40	0.4	22:30				2	•	1		40
10:45		4	18	4	18	36	22:45				2	8	2	4	12
11:00		3		4			23:00				6		0		
11:15		7		2			23:15				2		2		
11:30		5		6			23:30				3		1		
11:45		2	17	2	14	31	23:45				2	13	2	5	18
Tot o			119		113							180		108	
					113									100	
GPS Coordinates:	32,2707	30, -110.87	75540						NB	SB	i	Tot EB	S	WB	om inc
								_	IND	JD_					om ine
												299		221	
			M				_					M			
it			51.3%)	48.7%							62.5%		37.5%	
e our			07:30		08:15							15:15		16:00	
Jui														13.00	
														11	
o ume			33 0.52		25 0.78							21 0.88		16 0.67	

	•	•	1	†	ţ	1
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	ኻ	7	ሻ	^	† 1>	
Traffic Volume (veh/h)	25	38	53	1035	1983	74
Future Volume (veh/h)	25	38	53	1035	1983	74
Initial Q (Qb), veh	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00	1.00	1.00			1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach	No			No	No	
Adj Sat Flow, veh/h/ln	1870	1870	1870	1870	1870	1870
Adj Flow Rate, veh/h	43	66	62	1203	2228	83
Peak Hour Factor	0.58	0.58	0.86	0.86	0.89	0.89
Percent Heavy Veh, %	2	2	2	2	2	2
Cap, veh/h	109	97	205	2969	2584	96
Arrive On Green	0.06	0.06	0.04	0.84	0.74	0.74
Sat Flow, veh/h	1781	1585	1781	3647	3588	129
Grp Volume(v), veh/h	43	66	62	1203	1126	1185
Grp Sat Flow(s), veh/h/ln	1781	1585	1781	1777	1777	1847
Q Serve(g_s), s	2.0	3.6	0.6	7.3	39.3	40.7
Cycle Q Clear(g_c), s	2.0	3.6	0.6	7.3	39.3	40.7
Prop In Lane	1.00	1.00	1.00			0.07
Lane Grp Cap(c), veh/h	109	97	205	2969	1314	1366
V/C Ratio(X)	0.39	0.68	0.30	0.41	0.86	0.87
Avail Cap(c_a), veh/h	370	329	234	2969	1314	1366
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	1.00	1.00	1.00	1.00	1.00	1.00
Uniform Delay (d), s/veh	39.4	40.1	18.7	1.8	8.1	8.3
Incr Delay (d2), s/veh	2.3	8.0	0.8	0.4	7.4	7.7
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	0.9	3.3	1.0	1.2	13.3	14.3
Unsig. Movement Delay, s/veh						45.
LnGrp Delay(d),s/veh	41.7	48.1	19.6	2.2	15.5	15.9
LnGrp LOS	D	D	В	А	В	В
Approach Vol, veh/h	109			1265	2311	
Approach Delay, s/veh	45.6			3.0	15.7	
Approach LOS	D			А	В	
Timer - Assigned Phs		2		4	5	6
Phs Duration (G+Y+Rc), s		77.4		9.9	8.4	69.0
Change Period (Y+Rc), s		4.5		4.5	4.5	4.5
Max Green Setting (Gmax), s		72.9		18.1	5.3	63.1
Max Q Clear Time (q_c+l1), s		9.3		5.6	2.6	42.7
Green Ext Time (p_c), s		12.9		0.2	0.0	17.8
Intersection Summary						
HCM 6th Ctrl Delay			12.2			
HCM 6th LOS			12.2 B			
HOW OUT LOS			Ď			

	۶	→	•	•	←	•	1	†	/	/	↓	4
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	ሻ	- 11	7	ሽሽ	^	7	ሻ	^	7	ሻ		7
Traffic Volume (veh/h)	43	291	164	654	520	158	163	635	327	207	1332	43
Future Volume (veh/h)	43	291	164	654	520	158	163	635	327	207	1332	43
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach	1070	No	1070	1070	No	1070	1070	No	1070	1070	No	1070
Adj Sat Flow, veh/h/ln Adj Flow Rate, veh/h	1870 45	1870 303	1870 171	1870 696	1870 553	1870 168	1870 177	1870 690	1870 355	1870 252	1870 1624	1870 52
Peak Hour Factor	0.96	0.96	0.96	0.94	0.94	0.94	0.92	0.92	0.92	0.82	0.82	0.82
Percent Heavy Veh, %	0.70	2	2	2	2	2	2	2	2	2	2	2
Cap, veh/h	273	479	214	774	730	326	194	963	785	350	1275	568
Arrive On Green	0.15	0.13	0.13	0.22	0.21	0.21	0.11	0.27	0.27	0.20	0.36	0.36
Sat Flow, veh/h	1781	3554	1585	3456	3554	1585	1781	3554	1585	1781	3554	1585
Grp Volume(v), veh/h	45	303	171	696	553	168	177	690	355	252	1624	52
Grp Sat Flow(s), veh/h/ln	1781	1777	1585	1728	1777	1585	1781	1777	1585	1781	1777	1585
Q Serve(g_s), s	2.3	8.4	10.9	20.3	15.2	9.8	10.2	18.2	0.0	13.7	37.2	2.3
Cycle Q Clear(q_c), s	2.3	8.4	10.9	20.3	15.2	9.8	10.2	18.2	0.0	13.7	37.2	2.3
Prop In Lane	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Lane Grp Cap(c), veh/h	273	479	214	774	730	326	194	963	785	350	1275	568
V/C Ratio(X)	0.16	0.63	0.80	0.90	0.76	0.52	0.91	0.72	0.45	0.72	1.27	0.09
Avail Cap(c_a), veh/h	273	617	275	850	1292	576	194	963	785	350	1275	568
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Uniform Delay (d), s/veh	38.1	42.4	43.5	39.1	38.8	36.6	45.7	34.2	17.0	39.0	33.3	22.1
Incr Delay (d2), s/veh	0.3	1.4	12.1	11.8	1.6	1.3	40.8	4.6	1.9	7.0	129.6	0.3
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	1.0	3.7	4.9	9.6	6.6	3.8	6.5	8.1	5.6	6.4	37.8	0.9
Unsig. Movement Delay, s/veh LnGrp Delay(d),s/veh		42 O	55.6	50.9	40.4	37.9	04 E	20.0	10 0	14 O	162.9	22.4
LnGrp LOS	38.4 D	43.8 D	33.0 E	50.9 D	40.4 D	37.9 D	86.5 F	38.8 D	18.9 B	46.0 D	102.9 F	22.4 C
Approach Vol, veh/h	ע	519	<u> </u>	U	1417	ט	Г	1222	В	<u> </u>	1928	
Approach Delay, s/veh		47.2			45.3			39.9			143.8	
Approach LOS		47.2 D			45.5 D			37.7 D			F	
											'	
Timer - Assigned Phs	1	2	3	4	5	6	7	8				
Phs Duration (G+Y+Rc), s	24.9	32.6	27.7	18.5	15.8	41.7	20.4	25.8				
Change Period (Y+Rc), s	4.5	4.5	4.5	4.5	4.5	4.5	4.5	4.5				
Max Green Setting (Gmax), s	20.4	28.1	25.5	18.0	11.3	37.2	5.8	37.7				
Max Q Clear Time (g_c+l1), s	15.7	20.2	22.3	12.9	12.2	39.2	4.3	17.2				
Green Ext Time (p_c), s	0.3	3.4	0.9	1.1	0.0	0.0	0.0	4.1				
Intersection Summary												
HCM 6th Ctrl Delay			81.5									
HCM 6th LOS			F									

Intersection									
Int Delay, s/veh	0.5								
Movement	EBL	EBR	NBL	NBT	SBT	SBR			
Lane Configurations	7	7	7	44	ħβ				
Traffic Vol, veh/h	5	8	4	1110	2163	9			
Future Vol, veh/h	5	8	4	1110	2163	9			
Conflicting Peds, #/hr	0	0	0	0	0	0			
Sign Control	Stop	Stop	Free	Free	Free	Free			
RT Channelized	-	None	-	None	-	None			
Storage Length	0	0	200	-	-	-			
Veh in Median Storage	e, # 0	-	-	0	0	-			
Grade, %	0	-	-	0	0	-			
Peak Hour Factor	46	46	91	91	87	25			
Heavy Vehicles, %	2	2	2	2	2	2			
Mvmt Flow	11	17	4	1220	2486	36			
Major/Minor	Minor2	ľ	Major1	1	Major2				
Conflicting Flow All	3122	1261	2522	0		0			
Stage 1	2504	-	-	-	-	-			
Stage 2	618	-	-	-	-	-			
Critical Hdwy	6.84	6.94	4.14	-	-	-			
Critical Hdwy Stg 1	5.84	-	-	-	-	-			
Critical Hdwy Stg 2	5.84	-	-	-	-	-			
Follow-up Hdwy	3.52	3.32	2.22	-	-	-			
Pot Cap-1 Maneuver	~ 9	161	176	-	-	-			
Stage 1	47	-	-	-	-	-			
Stage 2	500	-	-	-	-	-			
Platoon blocked, %				-	-	-			
Mov Cap-1 Maneuver	~ 9	161	176	-	-	-			
Mov Cap-2 Maneuver	40	-	-	-	-	-			
Stage 1	46	-	-	-	-	-			
Stage 2	500	-	-	-	-	-			
a m g r									
Approach	EB		NB		SB				
HCM Control Delay, s	66.9		0.1		0				
HCM LOS	F								
Minor Lane/Major Mvn	nt	NBL	NBT	EBLn1 I	EBLn2	SBT	SBR		
Capacity (veh/h)		176		40	161		-		
HCM Lane V/C Ratio		0.025		0.272		_	-		
HCM Control Delay (s))	26		125.7	30.1	_	-		
HCM Lane LOS		D	_	123.7 F	D	_	-		
HCM 95th %tile Q(veh)	0.1	-	0.9	0.4	-	-		
	'/	0.1		0.7	0.4				
Notes									
~: Volume exceeds ca	pacity	\$: D€	elay exc	ceeds 3	00s	+: Com	putation Not Defined	*: All major volume in platoor	า

Intersection						
Int Delay, s/veh	0.2					
		EDD	///DI	WDT	NDI	NIDD
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	f	,	<u> </u>	↑	Ţ	7
Traffic Vol, veh/h	524	6	1	948	7	7
Future Vol, veh/h	524	6	1	948	7	7
Conflicting Peds, #/hr	0	0	0	0	0	0
	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None		
Storage Length	-	-	150	-	50	0
Veh in Median Storage,		-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	91	91	94	94	88	88
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	576	7	1	1009	8	8
Major/Minor M	aior1	N	Majora		Minor1	
	ajor1		Major2			F00
Conflicting Flow All	0	0	583	0	1591	580
Stage 1	-	-	-	-	580	-
Stage 2	-	-	-	-	1011	-
Critical Hdwy	-	-	4.12	-	6.42	6.22
Critical Hdwy Stg 1	-	-	-	-	5.42	-
Critical Hdwy Stg 2	-	-	-	-	5.42	-
Follow-up Hdwy	-	-	2.218	-	3.518	
Pot Cap-1 Maneuver	-	-	991	-	118	514
Stage 1	-	-	-	-	560	-
Stage 2	-	-	-	-	352	-
Platoon blocked, %	-	-		-		
Mov Cap-1 Maneuver	-	-	991	-	118	514
Mov Cap-2 Maneuver	-	-	-	-	118	-
Stage 1	-	-	-	-	560	-
Stage 2	-		_	_	352	
Jugo Z					332	
Approach	EB		WB		NB	
HCM Control Delay, s	0		0		24.9	
HCM LOS					С	
Minor Long/Major Mumat	N	UDI p1 I	VIDI 22	ГОТ	EDD	WDI
Minor Lane/Major Mvmt		VBLn1 N		EBT	EBR	WBL
Capacity (veh/h)		118	514	-	-	991
HCM Lane V/C Ratio		0.067		-	-	0.001
HCM Control Delay (s)		37.7	12.1	-	-	8.6
HCM Lane LOS		E	В	-	-	Α
HCM 95th %tile Q(veh)		0.2	0	-	-	0

	⋆	•	1	†	ļ	1
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	ች	7	ች	^	†	
Traffic Volume (veh/h)	60	50	47	1749	1008	44
Future Volume (veh/h)	60	50	47	1749	1008	44
Initial Q (Qb), veh	0	0	0	0	0	0
Ped-Bike Adj(A pbT)	1.00	1.00	1.00			1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00
Work one On Approach	No			No	No	
Adj Sat Flow, veh/h/ln	1870	1870	1870	1870	1870	1870
Adj Flow Rate, veh/h	103	86	55	2034	1133	49
Peak Hour Factor	0.58	0.58	0.86	0.86	0.89	0.89
Percent Heavy Veh, %	2	2	2	2	2	2
Cap, veh/h	458	408	339	2183	1656	72
Arrive On Green	0.26	0.26	0.07	0.61	0.48	0.48
Sat Flow, veh/h	1781	1585	1781	3647	3564	150
Grp Volume(v), veh/h	103	86	55	2034	580	602
Grp Sat Flow(s), veh/h/ln	1781	1585	1781	1777	1777	1843
Q Serve(g s), s	3.2	3.0	0.9	36.1	17.7	17.8
Cycle Q Clear(g c), s	3.2	3.0	0.9	36.1	17.7	17.8
Prop In Lane	1.00	1.00	1.00			0.08
Lane Grp Cap(c), veh/h	458	408	339	2183	848	880
V/C Ratio()	0.22	0.21	0.16	0.93	0.68	0.68
Avail Cap(c a), veh/h	458	408	339	2183	848	880
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	1.00	1.00	1.00	1.00	1.00	1.00
Uniform Delay (d), s/veh	20.5	20.4	9.6	12.2	14.2	14.2
Incr Delay (d2), s/veh	1.1	1.2	1.0	8.8	4.5	4.3
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	1.4	3.0	0.4	13.6	7.3	7.6
Unsig. Movement Delay, s/veh						
LnGrp Delay(d),s/veh	21.6	21.6	10.6	20.9	18.7	18.5
LnGrp LOS	С	С	В	С	В	В
Approach Vol, veh/h	189			2089	1182	
Approach Delay, s/veh	21.6			20.7	18.6	
Approach LOS	С			C	В	
		^				,
Timer - Assigned Phs		2		4	5	6
Phs Duration (G+ +Rc), s		47.5		22.5	9.6	37.9
Change Period (+Rc), s		4.5		4.5	4.5	4.5
Max Green Setting (Gmax), s		43.0		18.0	5.1	33.4
Max Q Clear Time (g c+l1), s		38.1		5.2	2.9	19.8
Green Ext Time (p c), s		4.4		0.4	0.0	6.7
Intersection Summary						
HCM 6th Ctrl Delay			20.0			
HCM 6th LOS			20.0 C			
HOW OUT LOS			C			

	۶	→	•	•	←	•	1	†	/	/	↓	✓
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	J.	^	7	14.14	^	7	Ť	^	7	T)	^	7
Traffic Volume (veh/h)	92	598	145	320	327	136	172	1215	506	220	841	32
Future Volume (veh/h)	92	598	145	320	327	136	172	1215	506	220	841	32
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A pbT)	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work one On Approach		No			No			No			No	
Adj Sat Flow, veh/h/ln	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870
Adj Flow Rate, veh/h	96	623	151	340	348	145	187	1321	550	268	1026	39
Peak Hour Factor	0.96	0.96	0.96	0.94	0.94	0.94	0.92	0.92	0.92	0.82	0.82	0.82
Percent Heavy Veh, %	2	2	2	2	2	2	2	2	2	2	2	2
Cap, veh/h	320	700	312	410	483	215	328	1167	708	276	1063	474
Arrive On Green	0.18	0.20	0.20	0.12	0.14	0.14	0.18	0.33	0.33	0.16	0.30	0.30
Sat Flow, veh/h	1781	3554	1585	3456	3554	1585	1781	3554	1585	1781	3554	1585
Grp Volume(v), veh/h	96	623	151	340	348	145	187	1321	550	268	1026	39
Grp Sat Flow(s),veh/h/ln	1781	1777	1585	1728	1777	1585	1781	1777	1585	1781	1777	1585
Q Serve(g s), s	4.2	15.3	7.6	8.6	8.4	7.8	8.6	29.4	15.7	13.4	25.5	1.6
Cycle Q Clear(g c), s	4.2	15.3	7.6	8.6	8.4	7.8	8.6	29.4	15.7	13.4	25.5	1.6
Prop In Lane	1.00		1.00	1.00		1.00	1.00	=	1.00	1.00	1010	1.00
Lane Grp Cap(c), veh/h	320	700	312	410	483	215	328	1167	708	276	1063	474
V/C Ratio()	0.30	0.89	0.48	0.83	0.72	0.67	0.57	1.13	0.78	0.97	0.96	0.08
Avail Cap(c a), veh/h	320	714	319	413	762	340	328	1167	708	276	1063	474
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Uniform Delay (d), s/veh	31.8	35.0	31.9	38.6	37.1	36.8	33.3	30.1	21.0	37.6	30.9	22.5
Incr Delay (d2), s/veh	0.5	13.2	1.2	13.2	2.0	3.6	2.3	70.7	8.2	45.6	20.3	0.3
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	1.8	7.7	2.9	4.3	3.7	3.2	3.7	22.9	5.9	9.0	13.0	0.6
Unsig. Movement Delay, s/veh		40.2	22.1	E1 0	20.1	10.1	35.6	100.0	20.1	ດາາ	51.3	22.9
LnGrp Delay(d),s/veh	32.4 C	48.2 D	33.1 C	51.8 D	39.1 D	40.4 D	33.0 D	100.8 F	29.1 C	83.2 F	51.3 D	22.9 C
LnGrp LOS Approach Vol, veh/h			<u> </u>	U	833	D	D			Г		
•		870			44.5			2058 75.7			1333 56.8	
Approach LOS		43.8			_			_			_	
Approach LOS		D			D			E			E	
Timer - Assigned Phs	1	2	3	4	5	6	7	8				
Phs Duration (G+ +Rc), s	18.4	33.9	15.1	22.1	21.0	31.3	20.6	16.7				
Change Period (+Rc), s	4.5	4.5	4.5	4.5	4.5	4.5	4.5	4.5				
Max Green Setting (Gmax), s	13.9	29.4	10.7	18.0	16.5	26.8	9.5	19.2				
Max Q Clear Time (g c+l1), s	15.4	31.4	10.6	17.3	10.6	27.5	6.2	10.4				
Green Ext Time (p c), s	0.0	0.0	0.0	0.4	0.2	0.0	0.1	1.8				
Intersection Summary												
HCM 6th Ctrl Delay			60.2									
HCM 6th LOS			Е									

Intersection									
Int Delay, s/veh	0.2								
Movement	EBL	EBR	NBL	NBT	SBT	SBR			
Lane Configurations	- 1	_ 7			ΦÞ				
Traffic Vol, veh/h	7	6	4	1876	1167	8			
Future Vol, veh/h	7	6	4	1876	1167	8			
Conflicting Peds, #/hr	0	0	0	0	0	0			
Sign Control	Stop	Stop	Free	Free	Free	Free			
RT Channelized	-	None	-	None	-	None			
Storage Length	0	0	200	-	-	-			
Veh in Median Storage	e, # 0	-	-	0	0	-			
Grade, %	0	-	-	0	0	-			
Peak Hour Factor	54	54	92	92	96	96			
Heavy Vehicles, %	2	2	2	2	2	2			
Mvmt Flow	13	11	4	2039	1216	8			
N 4 = i = = /N 4 i = 4 =	N //:		1-1-1		1-1				
	Minor2		/lajor1		Major2				
Conflicting Flow All	2248		1224	0	-	0			
Stage 1	1220	-	-	-	-	-			
Stage 2	1028	-	-	-	-	-			
Critical Hdwy	6.84	6.94	4.14	-	-	-			
Critical Hdwy Stg 1	5.84	-	-	-	-	-			
Critical Hdwy Stg 2	5.84	-	-	-	-	-			
Follow-up Hdwy	3.52	3.32	2.22	-	-	-			
Pot Cap-1 Maneuver	35	436	565	-	-	-			
Stage 1	242	-	-	-	-	-			
Stage 2	306	-	-	-	-	-			
Platoon blocked, %				-	-	-			
Mov Cap-1 Maneuver	35	436	565	-	-	-			
Mov Cap-2 Maneuver	138	-	-	-	-	-			
Stage 1	240	-	-	-	-	-			
Stage 2	306	-	-	-	-	-			
Approach	EB		NB		SB				
HCM Control Delay, s	24.4		0		0				
HCM LOS	C		- 3						
Minor Lane/Major Mvn	nt	NBL	NRT	EBLn1 I	FRI n2	SBT	SBR		
	IL								
Capacity (veh/h)		565	-	138	436	-	-		
HCM Cantral Dalay (a)		0.008		0.094		-	-		
HCM Control Delay (s)		11.4	-	33.8	13.5	-	-		
HCM Lane LOS	,	В	-	D	В	-	-		
HCM 95th %tile Q(veh)	0	-	0.3	0.1	-	-		

Int Delay, s/veh 0.2 Novement EBT EBR WBL WBT NBL NBR
Movement EBT EBR WBL WBT NBL NBR
Lane Configurations 1 1 1 Traffic Vol, veh/h 832 7 3 507 2 4 Future Vol, veh/h 832 7 3 507 2 4 Conflicting Peds, #/hr 0 0 0 0 0 0 0 Sign Control Free Free Free Free Stop Stop Stop RT Channelized - None - 0 0 - - 0 0 - - 0 0 - - 0 0 - - - - 0 <t< td=""></t<>
Traffic Vol, veh/h 832 7 3 507 2 4 Future Vol, veh/h 832 7 3 507 2 4 Conflicting Peds, #/hr 0 0 0 0 0 0 Sign Control Free Free Free Free Stop Stop RT Channelized - None - None - None Storage Length - - 150 - 50 0 Veh in Median Storage, # 0 - - 0 0 - Grade, % 0 - - 0 0 - Peak Hour Factor 90 90 92 92 50 50 Heavy Vehicles, % 2 2 2 2 2 2 2 Mymt Flow 924 8 3 551 4 8 Major/Minor Major1 Major2 Minor1 Conflicting Flow All 0 0 932 0 1485 928
Future Vol, veh/h 832 7 3 507 2 4 Conflicting Peds, #/hr 0 0 0 0 0 0 Sign Control Free Free Free Free Stop Stop RT Channelized - None - None - None Storage Length - - 150 - 50 0 Veh in Median Storage, # 0 - - 0 0 - Grade, % 0 - - 0 0 - Peak Hour Factor 90 90 92 92 50 50 Heavy Vehicles, % 2 2 2 2 2 2 2 Mymt Flow 924 8 3 551 4 8 Major/Minor Major/Minor Major
Conflicting Peds, #/hr 0 0 0 0 0 0 0 Sign Control Free Free Free Free Stop Stop RT Channelized - None - None - None Storage Length - - 150 - 50 0 Veh in Median Storage, # 0 - - 0 0 - Grade, % 0 - - 0 0 - Peak Hour Factor 90 90 92 92 50 50 Heavy Vehicles, % 2 2 2 2 2 2 2 2 Mvmt Flow 924 8 3 551 4 8 Major/Minor MajorI MajorI MajorI MajorI MinorI Conflicting Flow All O O 928 O 1485 928 Stage 1
Sign Control Free Free Free Free Stop Stop RT Channelized - None - None - None - None Storage Length 150 - 50 0 Veh in Median Storage, # 0 0 0 0 0 Grade, % 0 0 0 0 0 0 0 Peak Hour Factor 90 90 92 92 50 50 50 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0
RT Channelized - None - None - None Storage Length 150 - 50 0 Veh in Median Storage, # 0 0 0 - Grade, % 0 0 0 - Peak Hour Factor 90 90 92 92 50 50 Heavy Vehicles, % 2 2 2 2 2 2 2 2 Mvmt Flow 924 8 3 551 4 8 Major/Minor Major1 Major2 Minor1 Conflicting Flow All 0 0 932 0 1485 928 Stage 1 - 928 - 928 Stage 2 - 928 - 557 - Critical Hdwy - 4.12 - 6.42 6.22
Storage Length - - 150 - 50 0 Veh in Median Storage, # 0 - - 0 0 - Grade, % 0 - - 0 0 - Peak Hour Factor 90 90 92 92 50 50 Heavy Vehicles, % 2 2 2 2 2 2 2 Mvmt Flow 924 8 3 551 4 8 Major/Minor Major1 Major2 Minor1 Conflicting Flow All 0 0 932 0 1485 928 Stage 1
Veh in Median Storage, # 0 0 0 0 - Grade, % 0 0 0 0 - Peak Hour Factor 90 90 92 92 50 50 Heavy Vehicles, % 2 2 2 2 2 2 2 Mymt Flow 924 8 3 551 4 8 Major/Minor Major1 Major2 Minor1 Conflicting Flow All 0 932 0 1485 928 Stage 1 928 - Stage 2 557 - Critical Hdwy - 4.12 - 6.42 6.22
Grade, % 0 - - 0 0 - Peak Hour Factor 90 90 92 92 50 50 Heavy Vehicles, % 2 2 2 2 2 2 2 Mvmt Flow 924 8 3 551 4 8 Major/Minor Major1 Major2 Minor1 Conflicting Flow All 0 932 0 1485 928 Stage 1 - - - 928 - Stage 2 - - - 557 - Critical Hdwy - 4.12 - 6.42 6.22
Peak Hour Factor 90 90 92 92 50 50 Heavy Vehicles, % 2 2 2 2 2 2 2 Mvmt Flow 924 8 3 551 4 8 Major/Minor Major1 Major2 Minor1 Conflicting Flow All 0 932 0 1485 928 Stage 1 - - - 928 - Stage 2 - - - 557 - Critical Hdwy - 4.12 - 6.42 6.22
Heavy Vehicles, % 2 2 2 2 2 2 2 2 Mvmt Flow 924 8 3 551 4 8 Major/Minor Major1 Major2 Minor1 Conflicting Flow All 0 0 932 0 1485 928 Stage 1 928 - 557 - Critical Hdwy - 4.12 - 6.42 6.22
Mvmt Flow 924 8 3 551 4 8 Major/Minor Major1 Major2 Minor1 Conflicting Flow All 0 932 0 1485 928 Stage 1 - - - 928 - Stage 2 - - - 557 - Critical Hdwy - 4.12 - 6.42 6.22
Major/Minor Major1 Major2 Minor1 Conflicting Flow All 0 0 932 0 1485 928 Stage 1 - - - 928 - Stage 2 - - - 557 - Critical Hdwy - 4.12 - 6.42 6.22
Conflicting Flow All 0 0 932 0 1485 928 Stage 1 - - - 928 - Stage 2 - - - 557 - Critical Hdwy - 4.12 - 6.42 6.22
Conflicting Flow All 0 0 932 0 1485 928 Stage 1 - - - 928 - Stage 2 - - - 557 - Critical Hdwy - - 4.12 - 6.42 6.22
Conflicting Flow All 0 0 932 0 1485 928 Stage 1 - - - 928 - Stage 2 - - - 557 - Critical Hdwy - - 4.12 - 6.42 6.22
Stage 1 - - - 928 - Stage 2 - - - 557 - Critical Hdwy - - 4.12 - 6.42 6.22
Stage 2 557 - Critical Hdwy 4.12 - 6.42 6.22
Critical Hdwy 4.12 - 6.42 6.22
Critical Udun, Cta 1
Critical Hdwy Stg 1 5.42 -
Critical Hdwy Stg 2 5.42 -
Follow-up Hdwy 2.218 - 3.518 3.318
Pot Cap-1 Maneuver 734 - 137 325
Stage 1 385 -
Stage 2 574 -
Platoon blocked, %
Mov Cap-1 Maneuver 734 - 136 325
Mov Cap-2 Maneuver 136 -
Stage 1 385 -
Stage 2 572 -
Approach EB WB NB
HCM Control Delay, s 0 0.1 21.7
HCM LOS C
0
Minor Lane/Major Mvmt NBLn1 NBLn2 EBT EBR WBL WBT
Capacity (veh/h) 136 325 734 -
HCM Lane V/C Ratio 0.029 0.025 0.004 -
HCM Control Delay (s) 32.3 16.4 9.9 -
HCM Lane LOS D C A -
HCM 95th %tile Q(veh) 0.1 0.1 0 -

	⋆	•	1	†	ļ	1
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	*	7*	ች	^	† 1>	
Traffic Volume (veh/h)	26	40	55	1077	2063	77
Future Volume (veh/h)	26	40	55	1077	2063	77
Initial Q (Qb), veh	0	0	0	0	0	0
Ped-Bike Adj(A pbT)	1.00	1.00	1.00			1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00
Work one On Approach	No			No	No	
Adj Sat Flow, veh/h/ln	1870	1870	1870	1870	1870	1870
Adj Flow Rate, veh/h	45	69	64	1252	2318	87
Peak Hour Factor	0.58	0.58	0.86	0.86	0.89	0.89
Percent Heavy Veh, %	2	2	2	2	2	2
Cap, veh/h	111	99	188	3006	2648	99
Arrive On Green	0.06	0.06	0.04	0.85	0.76	0.76
Sat Flow, veh/h	1781	1585	1781	3647	3587	130
Grp Volume(v), veh/h	45	69	64	1252	1172	1233
Grp Sat Flow(s), veh/h/ln	1781	1585	1781	1777	1777	1847
Q Serve(q s), s	2.4	4.2	0.7	8.2	45.9	47.7
Cycle Q Clear(g c), s	2.4	4.2	0.7	8.2	45.9	47.7
Prop In Lane	1.00	1.00	1.00	J.E		0.07
Lane Grp Cap(c), veh/h	111	99	188	3006	1347	1400
V/C Ratio()	0.41	0.70	0.34	0.42	0.87	0.88
Avail Cap(c a), veh/h	329	293	209	3006	1347	1400
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	1.00	1.00	1.00	1.00	1.00	1.00
Uniform Delay (d), s/veh	44.2	45.0	23.2	1.8	8.4	8.6
Incr Delay (d2), s/veh	2.4	8.6	1.1	0.4	7.9	8.3
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.4	0.0	0.0
%ile BackOfQ(50%),veh/ln	1.1	0.0	1.2	1.5	15.7	17.0
Unsig. Movement Delay, s/veh		0.2	1.2	1.5	13.7	17.0
	46.6	53.6	24.3	2.2	16.3	16.9
LnGrp Delay(d),s/veh	40.0 D		24.3 C	2.2 A	10.3 B	16.9 B
LnGrp LOS		D	C			В
Approach Vol, veh/h	114			1316	2405	
Approach LOS	50.9			3.3	16.6	
Approach LOS	D			А	В	
Timer - Assigned Phs		2		4	5	6
Phs Duration (G+ +Rc), s		87.4		10.6	8.6	78.8
Change Period (+Rc), s		4.5		4.5	4.5	4.5
Max Green Setting (Gmax), s		82.9		18.1	5.3	73.1
Max Q Clear Time (g c+I1), s		10.2		6.2	2.7	49.7
Green Ext Time (p c), s		14.0		0.2	0.0	20.7
Intersection Summary						
HCM 6th Ctrl Delay			13.1			
HCM 6th LOS						
HOIVI OIII LUS			В			

	۶	→	•	•	←	•	1	†	~	/	↓	✓
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	J.	^	7	ሻሻ	^	7	ሻ	^	7	ሻ	^	7
Traffic Volume (veh/h)	45	303	171	680	541	164	170	661	340	215	1386	45
Future Volume (veh/h)	45	303	171	680	541	164	170	661	340	215	1386	45
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A pbT)	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work one On Approach		No			No			No			No	
Adj Sat Flow, veh/h/ln	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870
Adj Flow Rate, veh/h	47	316	178	723	576	174	185	718	370	262	1690	55
Peak Hour Factor	0.96	0.96	0.96	0.94	0.94	0.94	0.92	0.92	0.92	0.82	0.82	0.82
Percent Heavy Veh, %	2	2	2	2	2	2	2	2	2	2	2	2
Cap, veh/h	277	491	219	793	753	336	191	950	787	346	1257	561
Arrive On Green	0.16	0.14	0.14	0.23	0.21	0.21	0.11	0.27	0.27	0.19	0.35	0.35
Sat Flow, veh/h	1781	3554	1585	3456	3554	1585	1781	3554	1585	1781	3554	1585
Grp Volume(v), veh/h	47	316	178	723	576	174	185	718	370	262	1690	55
Grp Sat Flow(s),veh/h/ln	1781	1777	1585	1728	1777	1585	1781	1777	1585	1781	1777	1585
Q Serve(g s), s	2.4	8.8	11.5	21.4	16.0	10.2	10.9	19.5	0.0	14.6	37.2	2.4
Cycle Q Clear(g c), s	2.4	8.8	11.5	21.4	16.0	10.2	10.9	19.5	0.0	14.6	37.2	2.4
Prop In Lane	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Lane Grp Cap(c), veh/h	277	491	219	793	753	336	191	950	787	346	1257	561
V/C Ratio()	0.17	0.64	0.81	0.91	0.76	0.52	0.97	0.76	0.47	0.76	1.34	0.10
Avail Cap(c a), veh/h	277	608	271	838	1274	568	191	950	787	346	1257	561
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Uniform Delay (d), s/veh	38.5	42.9	44.0	39.5	39.0	36.7	46.7	35.4	17.4	40.0	34.0	22.7
Incr Delay (d2), s/veh	0.3	1.6	14.0	13.7	1.7	1.2	55.2	5.6	2.0	9.3	160.4	0.3
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	1.1	3.9	5.3	10.4	7.0	4.0	7.6	8.8	6.0	7.0	42.9	1.0
Unsig. Movement Delay, s/veh		44.5	F0.0	F0.0	40.7	07.0	101.0	44.0	10.4	40.4	1010	00.4
LnGrp Delay(d),s/veh	38.8	44.5	58.0	53.2	40.6	37.9	101.9	41.0	19.4	49.4	194.3	23.1
LnGrp LOS	D	D	E	D	D	D	F	D	В	D	F	С
Approach Vol, veh/h		541			1473			1273			2007	
Approach Delay, s/veh		48.4			46.5			43.6			170.7	
Approach LOS		D			D			D			F	
Timer - Assigned Phs	1	2	3	4	5	6	7	8				
Phs Duration (G+ +Rc), s	24.9	32.6	28.6	19.0	15.8	41.7	20.9	26.8				
Change Period (+Rc), s	4.5	4.5	4.5	4.5	4.5	4.5	4.5	4.5				
Max Green Setting (Gmax), s	20.4	28.1	25.5	18.0	11.3	37.2	5.8	37.7				
Max Q Clear Time (g c+I1), s	16.6	21.5	23.4	13.5	12.9	39.2	4.4	18.0				
Green Ext Time (p c), s	0.3	3.1	0.7	1.1	0.0	0.0	0.0	4.3				
Intersection Summary												
HCM 6th Ctrl Delay			93.1									
HCM 6th LOS			F									

Intersection										
Int Delay, s/veh	0.6									
Movement	EBL	EBR	NBL	NBT	SBT	SBR				
Lane Configurations	ř	7	ሻ	^	ħβ					
Traffic Vol, veh/h	5	8	4	1155	2250	9				
Future Vol, veh/h	5	8	4	1155	2250	9				
Conflicting Peds, #/hr	0	0	0	0	0	0				
Sign Control	Stop	Stop	Free	Free	Free	Free				
RT Channelized	-	None	-	None	-	None				
Storage Length	0	0	200	-	-	-				
Veh in Median Storage	e, # 0	-	-	0	0	-				
Grade, %	0	-	-	0	0	-				
Peak Hour Factor	46	46	91	91	87	25				
Heavy Vehicles, %	2	2	2	2	2	2				
Mvmt Flow	11	17	4	1269	2586	36				
Major/Minor	Minor2	N	Major1	ı	Major2					
Conflicting Flow All	3247	1311	2622	0	viajui z	0				
Stage 1	2604	1311	2022	-	-	-				
	643	-	-	-	-	-				
Stage 2	6.84	6.94	4.14		-					
Critical Hdwy	5.84	0.94	4.14	-		-				
Critical Hdwy Stg 1	5.84		-	-	-	-				
Critical Hdwy Stg 2		3.32	2.22	-	-	-				
Follow-up Hdwy	3.52 ~ 7	3.32 149	160	-						
Pot Cap-1 Maneuver			100	-	-	-				
Stage 1	41	-	-	-	-	-				
Stage 2	485	-	-	-	-	-				
Platoon blocked, %	7	1.40	1/0	-	-	-				
Mov Cap-1 Maneuver	~ 7	149	160	-	-	-				
Mov Cap-2 Maneuver		-	-	-	-	-				
Stage 1	40	-	-	-	-	-				
Stage 2	485	-	-	-	-	-				
Approach	EB		NB		SB					
HCM Control Delay, s	77.1		0.1		0					
HCM LOS	F									
Minor Lane/Major Mvn	nt	NBL	NBT	EBLn1 I	EBLn2	SBT	SBR			
Capacity (veh/h)		160	-	35	149	-				
HCM Lane V/C Ratio		0.027	_	0.311			-			
HCM Control Delay (s))	28.1		148.8	32.3	-	-			
HCM Lane LOS		D	_	F	D	_	-			
HCM 95th %tile Q(veh	1)	0.1	-	1	0.4	-	-			
•	.,	0.1			5. 1					
Notes										
~: Volume exceeds ca	pacity	\$: D∈	elay exc	ceeds 3	00s	+: Com	putation Not Defined	*: All major v	olume in platoc	on

Intersection							
Int Delay, s/veh	0.3						
		EDD	WDI	WDT	NDI	NDD	
Movement Configurations	EBT	EBR	WBL	WBT	NBL	NBR	
Lane Configurations	1	,	<u> </u>	^	`	7	
Traffic Vol, veh/h	546	6	1	986	8	8	
Future Vol, veh/h	546	6	1	986	8	8	
Conflicting Peds, #/hr	0	0	0	0	0	0	
Sign Control	Free	Free	Free	Free	Stop	Stop	
RT Channelized	-	None		None	-	None	
Storage Length	-	-	150	-	50	0	
Veh in Median Storage		-	-	0	0	-	
Grade, %	0	-	-	0	0	-	
Peak Hour Factor	91	91	94	94	88	88	
Heavy Vehicles, %	2	2	2	2	2	2	
Mvmt Flow	600	7	1	1049	9	9	
Major/Minor N	/lajor1		Major2	1	Minor1		
Conflicting Flow All	0	0	607	0	1655	604	
Stage 1	-	-		-	604	-	
Stage 2	-		-	-	1051	-	
Critical Hdwy	-	-	4.12	-	6.42	6.22	
Critical Hdwy Stg 1	-	-	4.12	-	5.42	0.22	
3 0	-	-	-	-	5.42	-	
Critical Hdwy Stg 2	-	-	2.218	-	3.518		
Follow-up Hdwy	-	-	971	-			
Pot Cap-1 Maneuver	-	-	9/1	-	108	498	
Stage 1	-	-	-	-	546	-	
Stage 2	-	-	-	-	336	-	
Platoon blocked, %	-	-	074	-	100	400	
Mov Cap-1 Maneuver	-	-	971	-	108	498	
Mov Cap-2 Maneuver	-	-	-	-	108	-	
Stage 1	-	-	-	-	546	-	
Stage 2	-	-	-	-	336	-	
Approach	EB		WB		NB		
HCM Control Delay, s	0		0		26.9		
HCM LOS	U		U		20.7 D		
TIOWI LOG					U		
Minor Lane/Major Mvm	t ſ	NBLn1 I	VBLn2	EBT	EBR	WBL	WBT
Capacity (veh/h)		108	498	-	-	971	-
HCM Lane V/C Ratio		0.084		-	-	0.001	-
HCM Control Delay (s)		41.4	12.4	-	-	8.7	-
HCM Lane LOS		Ε	В	-	-	Α	-
HCM 95th %tile Q(veh)		0.3	0.1	-	-	0	-

	⋆	•	1	†	ļ	1
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	ች	7	ች	^	†	
Traffic Volume (veh/h)	63	52	49	1820	1049	45
Future Volume (veh/h)	63	52	49	1820	1049	45
Initial Q (Qb), veh	0	0	0	0	0	0
Ped-Bike Adj(A pbT)	1.00	1.00	1.00			1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00
Work one On Approach	No			No	No	
Adj Sat Flow, veh/h/ln	1870	1870	1870	1870	1870	1870
Adj Flow Rate, veh/h	109	90	57	2116	1179	51
Peak Hour Factor	0.58	0.58	0.86	0.86	0.89	0.89
Percent Heavy Veh, %	2	2	2	2	2	2
Cap, veh/h	458	408	327	2183	1656	72
Arrive On Green	0.26	0.26	0.07	0.61	0.48	0.48
Sat Flow, veh/h	1781	1585	1781	3647	3564	150
Grp Volume(v), veh/h	109	90	57	2116	603	627
Grp Sat Flow(s), veh/h/ln	1781	1585	1781	1777	1777	1843
Q Serve(g s), s	3.4	3.1	1.0	39.7	18.8	18.8
Cycle Q Clear(g c), s	3.4	3.1	1.0	39.7	18.8	18.8
Prop In Lane	1.00	1.00	1.00			0.08
Lane Grp Cap(c), veh/h	458	408	327	2183	848	880
V/C Ratio()	0.24	0.22	0.17	0.97	0.71	0.71
Avail Cap(c a), veh/h	458	408	327	2183	848	880
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	1.00	1.00	1.00	1.00	1.00	1.00
Uniform Delay (d), s/veh	20.6	20.5	10.0	12.9	14.5	14.5
Incr Delay (d2), s/veh	1.2	1.2	1.2	13.3	5.0	4.9
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	1.5	3.2	0.4	16.1	7.8	8.1
Unsig. Movement Delay, s/veh						
LnGrp Delay(d),s/veh	21.8	21.7	11.1	26.1	19.5	19.4
LnGrp LOS	С	С	В	C	В	В
Approach Vol, veh/h	199			2173	1230	
Approach Delay, s/veh	21.8			25.8	19.5	
Approach LOS	C C			C C	В	
Timer - Assigned Phs		2		4	5	6
Phs Duration (G+ +Rc), s		47.5		22.5	9.6	37.9
Change Period (+Rc), s		4.5		4.5	4.5	4.5
Max Green Setting (Gmax), s		43.0		18.0	5.1	33.4
Max Q Clear Time (g c+I1), s		41.7		5.4	3.0	20.8
Green Ext Time (p c), s		1.2		0.4	0.0	6.6
Intersection Summary						
HCM 6th Ctrl Delay			23.4			
HCM 6th LOS			23.4 C			
HOW OUT LOS			C			

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Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	J.	^	7	14.14	44	7	ď	^	7	J.	^	7
Traffic Volume (veh/h)	96	622	151	333	340	141	179	1264	526	229	875	33
Future Volume (veh/h)	96	622	151	333	340	141	179	1264	526	229	875	33
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A pbT)	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work one On Approach		No			No			No			No	
Adj Sat Flow, veh/h/ln	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870
Adj Flow Rate, veh/h	100	648	157	354	362	150	195	1374	572	279	1067	40
Peak Hour Factor	0.96	0.96	0.96	0.94	0.94	0.94	0.92	0.92	0.92	0.82	0.82	0.82
Percent Heavy Veh, %	2	2	2	2	2	2	2	2	2	2	2	2
Cap, veh/h	319	711	317	411	496	221	327	1161	706	275	1058	472
Arrive On Green	0.18	0.20	0.20	0.12	0.14	0.14	0.18	0.33	0.33	0.15	0.30	0.30
Sat Flow, veh/h	1781	3554	1585	3456	3554	1585	1781	3554	1585	1781	3554	1585
Grp Volume(v), veh/h	100	648	157	354	362	150	195	1374	572	279	1067	40
Grp Sat Flow(s),veh/h/ln	1781	1777	1585	1728	1777	1585	1781	1777	1585	1781	1777	1585
Q Serve(g s), s	4.4	16.1	7.9	9.1	8.8	8.1	9.0	29.4	17.5	13.9	26.8	1.6
Cycle Q Clear(g c), s	4.4	16.1	7.9	9.1	8.8	8.1	9.0	29.4	17.5	13.9	26.8	1.6
Prop In Lane	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Lane Grp Cap(c), veh/h	319	711	317	411	496	221	327	1161	706	275	1058	472
V/C Ratio()	0.31	0.91	0.50	0.86	0.73	0.68	0.60	1.18	0.81	1.01	1.01	0.08
Avail Cap(c a), veh/h	319	711	317	411	758	338	327	1161	706	275	1058	472
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Uniform Delay (d), s/veh	32.1	35.2	32.0	38.9	37.1	36.8	33.7	30.3	21.6	38.0	31.6	22.8
Incr Delay (d2), s/veh	0.6	16.1	1.2	16.8	2.1	3.6	3.0	91.6	9.7	57.9	29.7	0.4
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	1.9	8.3	3.0	4.7	3.9	3.3	4.0	26.2	6.8	10.1	14.9	0.6
Unsig. Movement Delay, s/veh		54.0	00.0	FF 7	00.0	40.4	0/7	101.0	04.4	0/0	(1.0	00.4
LnGrp Delay(d),s/veh	32.7	51.3	33.2	55.7	39.2	40.4	36.7	121.9	31.4	96.0	61.3	23.1
LnGrp LOS	С	D	С	E	D	D	D	F	С	F	F	<u>C</u>
Approach Vol, veh/h		905			866			2141			1386	
Approach Delay, s/veh		46.1			46.1			90.0			67.2	
Approach LOS		D			D			F			E	
Timer - Assigned Phs	1	2	3	4	5	6	7	8				
Phs Duration (G+ +Rc), s	18.4	33.9	15.2	22.5	21.0	31.3	20.6	17.1				
Change Period (+Rc), s	4.5	4.5	4.5	4.5	4.5	4.5	4.5	4.5				
Max Green Setting (Gmax), s	13.9	29.4	10.7	18.0	16.5	26.8	9.5	19.2				
Max Q Clear Time (g c+I1), s	15.9	31.4	11.1	18.1	11.0	28.8	6.4	10.8				
Green Ext Time (p c), s	0.0	0.0	0.0	0.0	0.2	0.0	0.1	1.8				
Intersection Summary												
HCM 6th Ctrl Delay			69.4									
HCM 6th LOS			Е									

Interception						
Intersection Int Delay, s/veh	0.2					
J .						
	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	7	7	7	^	ተ ኈ	
Traffic Vol, veh/h	7	6	4	1952	1214	8
Future Vol, veh/h	7	6	4	1952	1214	8
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control S	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	0	200	-	-	-
Veh in Median Storage, #	0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	54	54	92	92	96	96
Heavy Vehicles, %	2	2	2	2	2	2
Mymt Flow	13	11	4	2122	1265	8
WWITHER	10		7	2122	1200	U
	nor2		/lajor1		Major2	
Conflicting Flow All 23	338	637	1273	0	-	0
Stage 1 12	269	-	-	-	-	-
Stage 2	069	-	-	-	-	-
Critical Hdwy 6	6.84	6.94	4.14	-	-	-
Critical Hdwy Stg 1 5	5.84	-	-	-	-	-
	5.84	-	-	-	-	-
	3.52	3.32	2.22	-	-	-
Pot Cap-1 Maneuver	31	420	541	-	-	-
	228	-		_	-	-
	291	_	-	-	-	_
Platoon blocked, %	-/1			_	_	_
Mov Cap-1 Maneuver	31	420	541	-	-	-
	130	420	J4 I	_	-	-
•		-	-	-	-	-
9	226	-	-	-	-	-
Stage 2	291	-	-	-	-	-
Approach	EB		NB		SB	
HCM Control Delay, s 2	25.6		0		0	
HCM LOS	D					
NA' 1 /NA ' NA 1		NDI	NDT	EDI 41	EDI 0	CDT
Minor Lane/Major Mvmt		NBL	MRII	EBLn1		SBT
Capacity (veh/h)		541	-	130	420	-
HCM Lane V/C Ratio		0.008	-		0.026	-
HCM Control Delay (s)		11.7	-	35.7	13.8	-
					_	
HCM Lane LOS HCM 95th %tile Q(veh)		В	-	0.3	B 0.1	-

Intercection						
Intersection Int Delay, s/veh	0.2					
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	Þ				7	7
Traffic Vol, veh/h	866	8	3	527	2	4
Future Vol, veh/h	866	8	3	527	2	4
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	-	150	-	50	0
Veh in Median Storage,	# 0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	90	90	92	92	50	50
Heavy Vehicles, %	2	2	2	2	2	2
Mymt Flow	962	9	3	573	4	8
WIVIII I IOW	702	,		010	7	0
	ajor1	1	Major2		Minor1	
Conflicting Flow All	0	0	971	0	1546	967
Stage 1	-	-	-	-	967	-
Stage 2	-	-	-	-	579	-
Critical Hdwy	-	-	4.12	-	6.42	6.22
Critical Hdwy Stg 1	-	_	-	-	5.42	-
Critical Hdwy Stg 2	-	-	_	-	5.42	-
Follow-up Hdwy	-	_	2.218	_	3.518	
Pot Cap-1 Maneuver	_		710	-	126	308
Stage 1	_		710	_	369	- 300
Stage 2	-	-	-		560	
	-	-	-	-	500	-
Platoon blocked, %	-	-	710	-	105	200
Mov Cap-1 Maneuver	-	-	710	-	125	308
Mov Cap-2 Maneuver	-	-	-	-	125	-
Stage 1	-	-	-	-	369	-
Stage 2	-	-	-	-	558	-
Approach	EB		WB		NB	
HCM Control Delay, s	0		0.1		22.9	
HCM LOS					С	
Minor Lane/Major Mvmt	1	NBLn11	VBLn2	EBT	EBR	WBL
Capacity (veh/h)		125	308	-	-	710
HCM Lane V/C Ratio		0.032		-		0.005
		34.8	17			10.1
HCM Long LOS				-	-	
HCM Lane LOS		D	C	-	-	В
HCM 95th %tile Q(veh)		0.1	0.1	-	-	0

	⋆	•	1	†	↓	1
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	ች	7	ች	^	† 1>	
Traffic Volume (veh/h)	26	40	55	1087	2071	77
Future Volume (veh/h)	26	40	55	1087	2071	77
Initial Q (Qb), veh	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00	1.00	1.00			1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach	No			No	No	
Adj Sat Flow, veh/h/ln	1870	1870	1870	1870	1870	1870
Adj Flow Rate, veh/h	45	69	64	1264	2327	87
Peak Hour Factor	0.58	0.58	0.86	0.86	0.89	0.89
Percent Heavy Veh, %	2	2	2	2	2	2
Cap, veh/h	111	99	186	3006	2648	98
Arrive On Green	0.06	0.06	0.04	0.85	0.76	0.76
Sat Flow, veh/h	1781	1585	1781	3647	3588	130
Grp Volume(v), veh/h	45	69	64	1264	1176	1238
Grp Sat Flow(s), veh/h/ln	1781	1585	1781	1777	1777	1847
Q Serve(g_s), s	2.4	4.2	0.7	8.3	46.5	48.2
Cycle Q Clear(q_c), s	2.4	4.2	0.7	8.3	46.5	48.2
Prop In Lane	1.00	1.00	1.00			0.07
Lane Grp Cap(c), veh/h	111	99	186	3006	1347	1400
V/C Ratio(X)	0.41	0.70	0.34	0.42	0.87	0.88
Avail Cap(c_a), veh/h	329	293	208	3006	1347	1400
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	1.00	1.00	1.00	1.00	1.00	1.00
Uniform Delay (d), s/veh	44.2	45.0	23.6	1.8	8.5	8.7
Incr Delay (d2), s/veh	2.4	8.6	1.1	0.4	8.1	8.5
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	1.1	0.2	1.2	1.5	16.0	17.3
Unsig. Movement Delay, s/veh						
LnGrp Delay(d),s/veh	46.6	53.6	24.7	2.2	16.6	17.2
LnGrp LOS	D	D	C	A	В	В
Approach Vol, veh/h	114			1328	2414	
Approach Delay, s/veh	50.9			3.3	16.9	
Approach LOS	D			Α	В	
Timer - Assigned Phs		2		4	5	6
Phs Duration (G+Y+Rc), s		87.4		10.6	8.6	78.8
Change Period (Y+Rc), s		4.5		4.5	4.5	4.5
Max Green Setting (Gmax), s		82.9		18.1	5.3	73.1
Max Q Clear Time (g_c+I1), s		10.3		6.2	2.7	50.2
Green Ext Time (p_c), s		14.2		0.2	0.0	20.3
Intersection Summary						
HCM 6th Ctrl Delay			13.2			
HCM 6th LOS			13.2 B			
HOW OUT LOS			ט			

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Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	ķ	^	7	14.14	44	7	J.	^	7	, j	^	7
Traffic Volume (veh/h)	46	304	176	683	541	164	174	662	342	215	1387	45
Future Volume (veh/h)	46	304	176	683	541	164	174	662	342	215	1387	45
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach		No			No			No			No	
Adj Sat Flow, veh/h/ln	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870
Adj Flow Rate, veh/h	48	317	183	727	576	174	189	720	372	262	1691	55
Peak Hour Factor	0.96	0.96	0.96	0.94	0.94	0.94	0.92	0.92	0.92	0.82	0.82	0.82
Percent Heavy Veh, %	2	2	2	2	2	2	2	2	2	2	2	2
Cap, veh/h	283	500	223	794	752	336	191	945	786	344	1251	558
Arrive On Green	0.16	0.14	0.14	0.23	0.21	0.21	0.11	0.27	0.27	0.19	0.35	0.35
Sat Flow, veh/h	1781	3554	1585	3456	3554	1585	1781	3554	1585	1781	3554	1585
Grp Volume(v), veh/h	48	317	183	727	576	174	189	720	372	262	1691	55
Grp Sat Flow(s),veh/h/ln	1781	1777	1585	1728	1777	1585	1781	1777	1585	1781	1777	1585
Q Serve(g_s), s	2.5	8.9	11.9	21.7	16.1	10.3	11.2	19.7	0.0	14.7	37.2	2.5
Cycle Q Clear(g_c), s	2.5	8.9	11.9	21.7	16.1	10.3	11.2	19.7	0.0	14.7	37.2	2.5
Prop In Lane	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Lane Grp Cap(c), veh/h	283	500	223	794	752	336	191	945	786	344	1251	558
V/C Ratio(X)	0.17	0.63	0.82	0.92	0.77	0.52	0.99	0.76	0.47	0.76	1.35	0.10
Avail Cap(c_a), veh/h	283	605	270	834	1268	566	191	945	786	344	1251	558
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Uniform Delay (d), s/veh	38.4	42.8	44.1	39.7	39.2	36.9	47.1	35.7	17.5	40.3	34.2	23.0
Incr Delay (d2), s/veh	0.3	1.6	15.3	14.2	1.7	1.2	62.8	5.8	2.0	9.6	163.5	0.4
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	1.1	4.0	5.5	10.5	7.1	4.0	8.1	8.9	6.1	7.1	43.3	1.0
Unsig. Movement Delay, s/veh		44.4	FO 4	F0.0	40.0	00.4	100.0	44.5	10 (F0.0	407.0	00.0
LnGrp Delay(d),s/veh	38.7	44.4	59.4	53.9	40.8	38.1	109.9	41.5	19.6	50.0	197.8	23.3
LnGrp LOS	D	D	E	D	D	D	F	D	В	D	F	С
Approach Vol, veh/h		548			1477			1281			2008	
Approach Delay, s/veh		48.9			46.9			45.2			173.7	
Approach LOS		D			D			D			F	
Timer - Assigned Phs	1	2	3	4	5	6	7	8				
Phs Duration (G+Y+Rc), s	24.9	32.6	28.8	19.4	15.8	41.7	21.3	26.9				
Change Period (Y+Rc), s	4.5	4.5	4.5	4.5	4.5	4.5	4.5	4.5				
Max Green Setting (Gmax), s	20.4	28.1	25.5	18.0	11.3	37.2	5.8	37.7				
Max Q Clear Time (g_c+I1), s	16.7	21.7	23.7	13.9	13.2	39.2	4.5	18.1				
Green Ext Time (p_c), s	0.3	3.1	0.6	1.0	0.0	0.0	0.0	4.3				
Intersection Summary												
HCM 6th Ctrl Delay			94.6									
HCM 6th LOS			F									

Intersection									
Int Delay, s/veh	2.7								
Movement	EBL	EBR	NBL	NBT	SBT	SBR			
Lane Configurations	¥	7	Ť	44	ħβ				
Traffic Vol, veh/h	13	16	14	1155	2250	18			
Future Vol, veh/h	13	16	14	1155	2250	18			
Conflicting Peds, #/hr	0	0	0	0	0	0			
Sign Control	Stop	Stop	Free	Free	Free	Free			
RT Channelized	-	None	-	None	-	None			
Storage Length	0	0	200	-	-	-			
Veh in Median Storage	e, # 0	-	-	0	0	-			
Grade, %	0	-	-	0	0	-			
Peak Hour Factor	46	46	91	91	87	25			
Heavy Vehicles, %	2	2	2	2	2	2			
Mvmt Flow	28	35	15	1269	2586	72			
Major/Minor	Minor2	N	Major1	ı	Major2				
Conflicting Flow All	3287	1329	2658	0	-	0			
Stage 1	2622	-	-	-	-	-			
Stage 2	665	-	-	-	-	-			
Critical Hdwy	6.84	6.94	4.14	-	-	-			
Critical Hdwy Stg 1	5.84	-	-	-	-	-			
Critical Hdwy Stg 2	5.84	-	-	-	-	-			
Follow-up Hdwy	3.52	3.32	2.22	-	-	-			
Pot Cap-1 Maneuver	~ 7	145	155	-	-	-			
Stage 1	40	-	-	-	-	-			
Stage 2	473	-	-	-	-	-			
Platoon blocked, %				-	-	-			
Mov Cap-1 Maneuver	~ 6	145	155	-	-	-			
Mov Cap-2 Maneuver	31	-	-	-	-	-			
Stage 1	36	-	-	-	-	-			
Stage 2	473	-	-	-	-	-			
Approach	EB		NB		SB				
HCM Control Delay, s	164.3		0.4		0				
HCM LOS	F								
Minor Lane/Major Mvn	nt	NBL	NBT	EBLn1 l	EBLn2	SBT	SBR		
Capacity (veh/h)		155	-	31	145	-	-		
HCM Lane V/C Ratio		0.099		0.912	0.24	-	-		
HCM Control Delay (s)	30.8		320.4	37.5	-	-		
HCM Lane LOS		D	- -	F	57.0 E	-	-		
HCM 95th %tile Q(veh	1)	0.3	-	3.1	0.9	-	-		
	,			J					
Notes		φ. Γ.	Jane		00-	0	autolian Nat Defin	* All	- ulata - u
~: Volume exceeds ca	pacity	\$: De	elay exc	eeds 3	UUS	+: Comp	putation Not Defined	*: All major volume in	n piatoon

Intersection							
Int Delay, s/veh	0.4						
		EDD	\//DI	\\/DT	NDI	NIDD	
Movement Lana Configurations	EBT	EBR	WBL	WBT	NBL	NBR	
Lane Configurations	þ	7	<u>ነ</u>	↑	10	7	
Traffic Vol. veh/h	551	7	1	990 990	10	10	
Future Vol, veh/h	551	7	1		10	10	
Conflicting Peds, #/hr	0 Eroo	0 Eroo	0 Eroo	0 Eroo	0 Stop	0 Stop	
Sign Control RT Channelized	Free	Free	Free	Free None	Stop	Stop	
	-	None	150		-	None	
Storage Length	- # 0	-	150	-	50	0	
Veh in Median Storage,		-	-	0	0	-	
Grade, %	0	- 01	- 04	0	0	-	
Peak Hour Factor	91	91	94	94	88	88	
Heavy Vehicles, %	2	2	2	2	2	2	
Mvmt Flow	605	8	1	1053	11	11	
Major/Minor M	lajor1		Major2		Minor1		
Conflicting Flow All	0	0	613	0	1664	609	
Stage 1	-	-	-	-	609	-	
Stage 2	-		-	-	1055	-	
Critical Hdwy	-	-	4.12	-	6.42	6.22	
Critical Hdwy Stg 1	-	-	4.12	-	5.42	0.22	
Critical Hdwy Stg 2	-	-	-	-	5.42	-	
	-	-	2.218		3.518		
Follow-up Hdwy	-	-	966	-			
Pot Cap-1 Maneuver	-	-		-	107	495	
Stage 1	-	-	-	-	543	-	
Stage 2	-	-	-	-	335	-	
Platoon blocked, %	-	-	0//	-	407	405	
Mov Cap-1 Maneuver	-	-	966	-	107	495	
Mov Cap-2 Maneuver	-	-	-	-	107	-	
Stage 1	-	-	-	-	542	-	
Stage 2	-	-	-	-	335	-	
Approach	EB		WB		NB		
HCM Control Delay, s	0		0		27.5		
HCM LOS	- 3		- 0		27.5 D		
HOW LOO					U		
		UDL 4	NIDL C	EST	ED.	14/51	
Minor Lane/Major Mvmt		VBLn1 N		EBT	EBR	WBL	
Capacity (veh/h)		107	495	-	-	966	
HCM Lane V/C Ratio		0.106		-	-	0.001	
						0.7	
HCM Control Delay (s)		42.6	12.4	-	-	8.7	
HCM Control Delay (s) HCM Lane LOS HCM 95th %tile Q(veh)		42.6 E 0.3	12.4 B 0.1	-	-	8.7 A 0	

	•	•	1	†	ţ	1
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	*	7	ች	^	†	
Traffic Volume (veh/h)	63	52	49	1831	1059	45
Future Volume (veh/h)	63	52	49	1831	1059	45
Initial Q (Qb), veh	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00	1.00	1.00			1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach	No			No	No	
Adj Sat Flow, veh/h/ln	1870	1870	1870	1870	1870	1870
Adj Flow Rate, veh/h	109	90	57	2129	1190	51
Peak Hour Factor	0.58	0.58	0.86	0.86	0.89	0.89
Percent Heavy Veh, %	2	2	2	2	2	2
Cap, veh/h	458	408	324	2183	1657	71
Arrive On Green	0.26	0.26	0.07	0.61	0.48	0.48
Sat Flow, veh/h	1781	1585	1781	3647	3565	149
Grp Volume(v), veh/h	109	90	57	2129	609	632
Grp Sat Flow(s), veh/h/ln	1781	1585	1781	1777	1777	1844
Q Serve(g_s), s	3.4	3.1	1.0	40.3	19.1	19.1
Cycle Q Clear(g_c), s	3.4	3.1	1.0	40.3	19.1	19.1
Prop In Lane	1.00	1.00	1.00			0.08
Lane Grp Cap(c), veh/h	458	408	324	2183	848	880
V/C Ratio(X)	0.24	0.22	0.18	0.98	0.72	0.72
Avail Cap(c_a), veh/h	458	408	324	2183	848	880
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	1.00	1.00	1.00	1.00	1.00	1.00
Uniform Delay (d), s/veh	20.6	20.5	10.1	13.0	14.6	14.6
Incr Delay (d2), s/veh	1.2	1.2	1.2	14.3	5.2	5.0
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	1.5	3.2	0.4	16.6	8.0	8.2
Unsig. Movement Delay, s/veh						
LnGrp Delay(d),s/veh	21.8	21.7	11.2	27.2	19.7	19.6
LnGrp LOS	С	С	В	С	В	В
Approach Vol, veh/h	199			2186	1241	
Approach Delay, s/veh	21.8			26.8	19.7	
Approach LOS	C C			C C	В	
Timer - Assigned Phs		2		4	5	6
Phs Duration (G+Y+Rc), s		47.5		22.5	9.6	37.9
Change Period (Y+Rc), s		4.5		4.5	4.5	4.5
Max Green Setting (Gmax), s		43.0		18.0	5.1	33.4
Max Q Clear Time (g_c+I1), s		42.3		5.4	3.0	21.1
Green Ext Time (p_c), s		0.6		0.4	0.0	6.6
Intersection Summary						
HCM 6th Ctrl Delay			24.1			
HCM 6th LOS			24.1 C			
HOW OUT LOS			C			

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Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	ሻ	^	7	ሻሻ	^	7	ሻ	^	7	7	^	7
Traffic Volume (veh/h)	96	623	157	337	341	141	184	1265	529	229	876	34
Future Volume (veh/h)	96	623	157	337	341	141	184	1265	529	229	876	34
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach		No			No			No			No	
Adj Sat Flow, veh/h/ln	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870
Adj Flow Rate, veh/h	100	649	164	359	363	150	200	1375	575	279	1068	41
Peak Hour Factor	0.96	0.96	0.96	0.94	0.94	0.94	0.92	0.92	0.92	0.82	0.82	0.82
Percent Heavy Veh, %	2	2	2	2	2	2	2	2	2	2	2	2
Cap, veh/h	319	711	317	411	497	222	327	1161	706	275	1058	472
Arrive On Green	0.18	0.20	0.20	0.12	0.14	0.14	0.18	0.33	0.33	0.15	0.30	0.30
Sat Flow, veh/h	1781	3554	1585	3456	3554	1585	1781	3554	1585	1781	3554	1585
Grp Volume(v), veh/h	100	649	164	359	363	150	200	1375	575	279	1068	41
Grp Sat Flow(s), veh/h/ln	1781	1777	1585	1728	1777	1585	1781	1777	1585	1781	1777	1585
Q Serve(g_s), s	4.4	16.1	8.3	9.2	8.8	8.1	9.3	29.4	17.7	13.9	26.8	1.7
Cycle Q Clear(g_c), s	4.4	16.1	8.3	9.2	8.8	8.1	9.3	29.4	17.7	13.9	26.8	1.7
Prop In Lane	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Lane Grp Cap(c), veh/h	319	711	317	411	497	222	327	1161	706	275	1058	472
V/C Ratio(X)	0.31	0.91	0.52	0.87	0.73	0.68	0.61	1.18	0.81	1.01	1.01	0.09
Avail Cap(c_a), veh/h	319	711	317	411	758	338	327	1161	706	275	1058	472
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Uniform Delay (d), s/veh	32.1	35.2	32.1	39.0	37.1	36.8	33.8	30.3	21.7	38.0	31.6	22.8
Incr Delay (d2), s/veh	0.6	16.2	1.5	18.4	2.1	3.6	3.4	92.0	10.0	57.9	30.0	0.4
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	1.9	8.3	3.2	4.8	3.9	3.3	4.1	26.2	6.9	10.1	14.9	0.7
Unsig. Movement Delay, s/veh		F1 F	22 /	F7 4	20.1	40.0	27.2	100.0	21.7	0/ 0	/1 /	22.1
LnGrp Delay(d),s/veh	32.7	51.5	33.6	57.4	39.1	40.3	37.2	122.3	31.7	96.0	61.6	23.1
LnGrp LOS	С	D 012	С	E	D 072	D	D	F 2150	С	F	F 1200	С
Approach Vol, veh/h		913			872			2150			1388	
Approach LOS		46.2			46.9			90.1			67.3	
Approach LOS		D			D			F			E	
Timer - Assigned Phs	1	2	3	4	5	6	7	8				
Phs Duration (G+Y+Rc), s	18.4	33.9	15.2	22.5	21.0	31.3	20.6	17.1				
Change Period (Y+Rc), s	4.5	4.5	4.5	4.5	4.5	4.5	4.5	4.5				
Max Green Setting (Gmax), s	13.9	29.4	10.7	18.0	16.5	26.8	9.5	19.2				
Max Q Clear Time (g_c+I1), s	15.9	31.4	11.2	18.1	11.3	28.8	6.4	10.8				
Green Ext Time (p_c), s	0.0	0.0	0.0	0.0	0.2	0.0	0.1	1.8				
Intersection Summary												
HCM 6th Ctrl Delay			69.6									
HCM 6th LOS			Е									

Intersection									
Int Delay, s/veh	0.6								
Movement	EBL	EBR	NBL	NBT	SBT	SBR			
Lane Configurations	7	7	- 1	^	Φ₽				
Traffic Vol, veh/h	16	16	15	1952	1214	19			
Future Vol, veh/h	16	16	15	1952	1214	19			
Conflicting Peds, #/hr	0	0	0	0	0	0			
Sign Control	Stop	Stop	Free	Free	Free	Free			
RT Channelized	-	None	-	None	-	None			
Storage Length	0	0	200	-	-	-			
Veh in Median Storage	e, # 0	-	-	0	0	-			
Grade, %	0	-	-	0	0	-			
Peak Hour Factor	54	54	92	92	96	96			
Heavy Vehicles, %	2	2	2	2	2	2			
Mvmt Flow	30	30	16	2122	1265	20			
Major/Minor	Minor2	N	/lajor1	N	Major2				
Conflicting Flow All	2368	643	1285	0	-	0			
Stage 1	1275	-	-	-		-			
Stage 2	1093	_	_	_	_	_			
Critical Hdwy	6.84	6.94	4.14	-		-			
Critical Hdwy Stg 1	5.84	0.71		_		_			
Critical Hdwy Stg 2	5.84	-	_	_	_	_			
Follow-up Hdwy	3.52	3.32	2.22	_	_	_			
Pot Cap-1 Maneuver	~ 29	416	536	_	_	_			
Stage 1	226	- 10	-	_		_			
Stage 2	283	-	_	-		-			
Platoon blocked, %	200			_	_	_			
Mov Cap-1 Maneuver	~ 28	416	536	-		-			
Mov Cap 1 Maneuver	122	- 10	-	_	_	_			
Stage 1	219	_	_	_	_	_			
Stage 2	283	_	_	_	_	_			
Stage 2	200								
Approach	EB		NB		SB				
HCM Control Delay, s	29		0.1		0				
HCM LOS	D		0.1		- 0				
HOW LOS	U								
Minor Lane/Major Mvn	nt	NBL	NBT	EBLn1 l	FBI n2	SBT	SBR		
Capacity (veh/h)		536	-	122	416		-		
HCM Lane V/C Ratio		0.03		0.243		_	-		
HCM Control Delay (s))	11.9	-	43.7	14.3	-	-		
HCM Lane LOS		В	-	43.7 E	14.3 B	-	-		
HCM 95th %tile Q(veh)	0.1	-	0.9	0.2	-	-		
	7	0.1		0.7	0.2				
Notes		A 5			20		1 11 N 1 D 61 1	* All	
~: Volume exceeds ca	pacity	\$: De	lay exc	eeds 30	JUS	+: Com	putation Not Defined	*: All major volume in platoon	

Intersection						
Int Delay, s/veh	0.3					
		EDD.	WDL	WDT	NDI	NDD
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	f	10	<u> </u>	†	Ť	_ ř
Traffic Vol, veh/h	872	10	5	532	4	5
Future Vol, veh/h	872	10	5	532	4	5
Conflicting Peds, #/hr	0	_ 0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	
Storage Length	-	-	150	-	50	0
Veh in Median Storage,		-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	90	90	92	92	50	50
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	969	11	5	578	8	10
Major/Minor	010-1		Anie 2		Aine -1	
	ajor1		Major2		Minor1	
Conflicting Flow All	0	0	980	0	1563	975
Stage 1	-	-	-	-	975	-
Stage 2	-	-	-	-	588	-
Critical Hdwy	-	-	4.12	-	6.42	6.22
Critical Hdwy Stg 1	-	-	-	-	5.42	-
Critical Hdwy Stg 2	-	-	-	-	5.42	-
Follow-up Hdwy	-	-	2.218	-	3.518	3.318
Pot Cap-1 Maneuver	-	-	704	-	123	305
Stage 1	-	-	-	-	366	-
Stage 2	-	-	-	-	555	-
Platoon blocked, %	-	-		-		
Mov Cap-1 Maneuver	-	-	704	-	122	305
Mov Cap-2 Maneuver	-		-	-	122	-
Stage 1	-	-	-	-	363	-
Stage 2	_		_	_	555	_
Jiage Z	-	-	-	-	333	-
Approach	EB		WB		NB	
HCM Control Delay, s	0		0.1		25.8	
HCM LOS					D	
NA!		UDL 4.	IDL C	EDT	CD.	MDI
Minor Lane/Major Mvmt		VBLn1 N		EBT	EBR	WBL
Capacity (veh/h)		122	305	-	-	704
HCM Lane V/C Ratio		0.066		-	-	0.008
HCM Control Delay (s)		36.6	17.2	-	-	10.2
HCM Lane LOS		Е	С	-	-	В
HCM 95th %tile Q(veh)		0.2	0.1	-	-	0